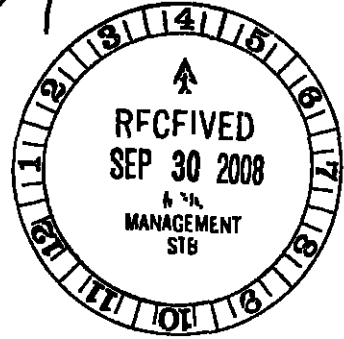


223704

**BEFORE THE  
SURFACE TRANSPORTATION BOARD**

**STB FINANCE DOCKET NO. 35160**



**OREGON INTERNATIONAL PORT OF COOS BAY  
—FEEDER LINE APPLICATION—  
COOS BAY LINE  
OF THE CENTRAL OREGON & PACIFIC RAILROAD, INC.**

**SUPPLEMENTAL REPLY OF THE  
OREGON INTERNATIONAL PORT OF COOS BAY**

**Exhibit 3**

**Bridge Report**

**ENTERED  
Office of Proceedings  
OCT - 1 2008  
Part of  
Public Record**



**DAVID EVANS  
AND ASSOCIATES INC**

September 26, 2008

Mr. Michael Gaul  
Port of Coos Bay  
P. O. Box 1215  
Coos Bay, OR 97420

**SUBJECT: COOS BAY RAIL LINK (LINE) BRIDGE CONDITION ASSESSMENT**

Dear Mike

Enclosed is the Final Bridge Condition Assessment Report for the Coos Bay Rail Link in accordance with Task 5 of our contract. We have completed a review of the 2007 Osmose Railroad Services, Inc. Bridge Inspection Reports and conducted visual inspections of the bridges on the Line. We have developed opinions of anticipated bridge costs associated with resuming rail operations, along with short and long-term bridge system costs and costs to remove the bridge system and replace all bridges with new structures. This report documents our study methodology and findings.

As you know, David Evans and Associates, Inc. (DEA) is a national leader in sustainable design and management solutions, and has consistently provided its clients with award-winning approaches to transportation, energy, water resources, and land development design, planning, and management. As a result, the company has consistently ranked among Engineering News Record's Top 100 Pure Design firms in the U.S. and among the leaders in many of its local markets.

We have extensive experience in transportation projects, especially in Oregon, and have the capability to provide design-build services for our projects. In July 2008, the ODOT Rail Division retained our firm to prepare a Bridge Condition Study on 331 significant (length greater than 100 feet) short line railroad bridges, which included bridges from nearly all short lines in the state. Our cost estimates are based upon our extensive experience with bridge work in the Pacific Northwest, as well as experience derived from ODOT Rail's Bridge Condition Study.

In the last two years, we have successfully delivered design and construction engineering services for six Coos County bridges. We are a recognized leader for steel truss bridge rehabilitation in Oregon and have most recently been selected to provide professional design and construction engineering services for the rehabilitation of the Coos Bay Railroad Bridge, which is a significant steel truss bridge. We have a unique understanding of the cost of construction in Coos County. For more about DEA, please visit our web site at [www.deainc.com](http://www.deainc.com).

If you have any questions, please call me at (503) 361-8635.

Sincerely,

**DAVID EVANS AND ASSOCIATES, INC.**

Jeff Parker, P.E.  
Project Manager

cc: Sandra Brown, Troutman Sanders LLP

# Coos Bay Rail Link Bridge Condition Assessment



**Prepared for:**

**Oregon International Port of Coos Bay**

**September 26, 2008**

**Prepared by:**



**DAVID EVANS  
AND ASSOCIATES INC**

# **Coos Bay Rail Link Bridge Condition Assessment Final Report**

## **Project Background**

The Oregon International Port of Coos Bay (Port) is seeking to acquire the Coos Bay Rail Link (Line) from the Central Oregon & Pacific Railroad, Inc (CORP). The location of the Line is between milepost 652 114, near Danebo, OR and milepost 763 130, near Cordes, OR. The Port retained David Evans and Associates, Inc (DEA) to perform a condition assessment (Study) of the bridges along the Line. The findings of this Study are included and summarized in this report, to be submitted to the Port on September 29, 2008.

## **Purpose and Need for Report**

The objective of this report is to provide opinions of costs to bring the 107 bridges located along the Line (reference Attachment 4 for bridge location maps) to a condition that would allow the Line to reopen as well as anticipated short-term and anticipated long-term costs of the bridge system associated with owning and operating the Line.

The Study is based upon a visual assessment of the bridges, quantification of repairs based on recommendations noted in bridge inspection reports completed by Osmose Railroad Services, Inc (Osmose) in 2007, and development of an opinion of probable bridge system costs which a new owner may encounter. Three costs for each bridge have been evaluated—those anticipated to open the Line to traffic (immediate), those anticipated to occur over for the next 5 years (short-term), and those anticipated to occur beyond 5 years (long-term).

## **Existing Information**

### **Rail Line Location and History**

The following brief narrative is provided for the user of this report to understand, in context, the approximate age of the bridge structures studied and ownership since construction. The Line is approximately 111 miles long and located entirely in Oregon, while traversing portions of Coos, Douglas and Lane Counties.

The Southern Pacific Railroad (SPRR) opened the Line from Eugene to Marshfield (now known as Coos Bay) in 1916. The section of the Line from Eugene to Coos Bay is the proposed Coos Bay Rail Link. SPRR sold the Line on December 31, 1994 to CORP.

The existing bridge system was designed and constructed for a Cooper E-50 – E-60 design load. This load carrying design capacity is  $\frac{3}{4}$  of a new rail bridge designed today.

### **Osmose Railroad Services, Inc. (Osmose) Bridge Inspection Reports**

The Port provided to the Study Team the Bridge Inspection Reports completed by Osmose in 2005 and 2007. Due to the large size of these documents, only the 2007 reports are bound in this report for the Port as Attachment 1 and we understand that the 2007 and 2005 Osmose Reports were included in Volume III and IV respectively of Reply of the Oregon International Port of Coos Bay, filed September 12, 2008.

DEA has reviewed existing Bridge Inspection Reports completed by Osmose in 2007 and tabulated prioritized repairs noted in those reports, which were provided by the Port of Coos Bay.

# **Coos Bay Rail Link Bridge Condition Assessment Final Report**

## **Study Organization and Methodology**

The primary elements of the Study are

- Existing Osmose data collection and tabulation
- *Bridge Inspections*
- Development of opinions of cost for recommended repairs

Each of these Study activities is described below

### **Existing Osmose Data Collection and Tabulation**

The 2005 and 2007 Osmose Bridge Inspection Reports summarized and prioritized recommended repair work for each bridge. This repair work was quantified from both of the inspection reports for each bridge. From this work, the Study Team was able to identify the repairs that were recommended in 2007 but not noted for 2005. Note that much of the work identified in the 2005 report as Priority 2 and 3 remains on the 2007 report as maintenance deferred. By examination of repair work completed in the 2007 Osmose reports, we estimate approximately less than 1% of the repairs recommended by Osmose in 2005 were completed by the time the 2007 Osmose inspections were conducted. We understand that in November 2007, CORP put forward a cost estimate for Phase I bridge repairs of \$6.75 million, which appears to be based on priority 2 repairs, and a cost estimate for Phase II bridge repairs of \$3.75 million, which appears to be based on priority 3 repairs. We have been told by the Port's counsel that no detail was provided for these estimates and that the back-up for these estimates was not provided to the Port in response to discovery served on CORP. As noted below, DEA has provided summary and detailed estimates for the repairs needed for these bridges by priority based upon 2009 dollars.

### **Bridge Inspections**

The Study Team conducted visual inspections of the bridges (with the exception of Bridge No. 716.40) on the Line from September 13th to September 18th. Field reports for the 2008 DEA Visual Inspections are included in Attachment 2 of this report. In addition, the Study Team conducted an annual inspection of Bridge No. 716.40 and this report is included as Attachment 5 of this report.

The following narrative has been included for an understanding of the level of review involved for three general categories of bridge inspection:

1. A visual inspection is the basic inspection and is conducted for observation review; no material tests are performed. The observations take place from the bridge deck and/or areas accessed by foot below. For this Study, visual inspections were conducted to review the Osmose routine Bridge Inspection Reports and to identify obvious additional damage which may have occurred since the Line was embargoed. In addition, this visual inspection was conducted to correlate the Study Team's understanding of the bridges' conditions and general state of maintenance with the repair recommendations contained in the Osmose report.
2. An annual or biannual routine inspection builds on the visual inspection and is generally the industry standard inspection for routine inspections. A structural member condition inventory is assembled for each structural element (beam, column, pile, cap, etc.).
3. A detailed inspection represents the next level of inspection and is generally conducted prior to a major bridge rehabilitation or in order to establish a safe load capacity. A detailed inspection includes the evaluation of each structural element for its condition, which is a more in-depth, time-intensive inspection than a routine inspection and can include supplemental equipment to include ladders, mechanical lifts, and boats. A detailed inspection can include limited material testing, including limited boring of representative timber members to establish if any interior deterioration is occurring; and for steel, the actual steel thickness is measured to determine the remaining structural section. A structural member condition inventory is assembled for each structural element (beam, column, pile, cap, etc.). This inspection includes a load capacity analysis based upon

## **Coos Bay Rail Link Bridge Condition Assessment Final Report**

the remaining structural section for each member. Calculations and a summary of the load capacity are submitted in a load rating report.

None of the inspection categories include underwater inspections unless scour or other problems are noted. Both the routine and detailed inspections include an itemization of repairs for each bridge. Our teams also prepared itemized repair recommendations and costs for all visually inspected bridges based upon the 2007 Osmose inspection report and any additional data gathered during the visual inspection of each bridge.

### **Cost Estimating**

#### **Osmose Bridge Repair Recommendations**

The Osmose Bridge Inspection Reports prioritize recommended bridge repair work based on the following designations, which we have also used in our report:

***Priority 1 – Emergency:*** Stop operation over the structure and perform repairs immediately. There are no Priority 1 repairs noted in the 2007 reports. The visual inspections by DEA did not find new Priority 1 repairs required for bridges, however, due to ship/bridge unreported accidents, scour associated with high flow events, debris collisions with bridge elements, etc. which may occur prior to opening the Line, we recommend the new rail operator conduct a visual track and bridge inspection immediately prior to opening the Line. Also, during our visual inspection, slides were noted along the rail Line that stop operations and would need to be cleared and stabilized prior to opening the Line.

***Priority 2 – Condition is unsafe and could cause failure at any time:*** Repair as soon as possible. Condition must be monitored continually until repairs have been completed. Repairs classified as Priority 2 are recommended to be complete prior to opening the Line to rail traffic.

***Priority 3 – Condition could become unsafe and should be monitored frequently:*** Repair in the near future.

***Priority 4 – Condition is substandard and should be monitored:*** Condition may require repairs within one to five years.

***Priority 5 – Either no defects or only minor defects were found:*** Repairs not recommended at this time, but condition of structure should be monitored.

#### **Rail Operation Considerations Influencing Bridge Costs**

The following discussion relates to the costs to immediately open the bridge system to traffic, as well as short- and long-term bridge system costs and how they are influenced heavily by the intended operational plans for the Line. If the use of the Line changes and additional capacity (heavier cars and faster speeds) is required, the cost to maintain and improve the Line increases. All costs noted in this section (Priority 2, 3, 4, Rehabilitation, Replacement, and Removal) are noted in Figures 1 and 2 located in the Summary of this report. More detailed calculations for these costs are contained in Attachment 3 of this report.

A railroad with limited freight, seeking to maintain the Line with minimal expenditures, will keep operating with the rail Line under the conditions which existed prior to the embargo, i.e. E-60 maximum loading, slow speeds less than 10 mph on bridges, all rail cars loaded to meet the E-60 loading capacity, and spacing of cars to avoid having maximum loaded cars adjacent to each other. The initial cost to get this bridge system up and running under the operating conditions in place prior to the embargo is the cost of the Priority 2 repairs. In addition, this railroad bridge system may cost approximately \$1,200,000 annually to correct new Priority 2 repairs identified during annual bridge inspections. However, there will also be unanticipated and indeterminate expenditures. This amount is influenced by the continued past practice of deferred maintenance (Priority 3 and 4), which delayed maintenance until they become a Priority 1 or 2. This is the primary assumption for development of costs to begin operations, as well as short-term and long-term bridge costs identified in this report. This is the assumption used by railroads to operate a line for profit, with limited revenue or freight. This method of management of a bridge

## **Coos Bay Rail Link Bridge Condition Assessment Final Report**

system can be maintained indefinitely. However, with this method, at some point in time, the continued deterioration of the system will yield unexpected and relatively large expenditures to correct nearly 100 years of deferred maintenance. When this time will arrive is not known. However, our opinion is that the largest expenditures will occur when currently designated Priority 3 and 4 repairs for the steel truss spans in the system (especially the Siuslaw and Umpqua River) are deemed by future bridge inspectors to be Priority 1 or 2. When this occurs, in order to maintain operations, the Port can expect large unplanned expenditures, which are impossible to fully quantify or schedule. We cannot determine if this point is 5, 10 or 50 years in the future. The only way to avoid unanticipated expenditures of a bridge system this age is to proactively invest in the bridges to arrest deterioration and restore the condition of the bridge system by reversing several decades of deferred maintenance.

A railroad with a plan to gradually incorporate improvements to upgrade the Line to E-80 standards, while maintaining current operations at the E-60 maximum, would slowly upgrade the bridge system with the average speed being 30 mph. The cars would still be loaded to meet the E-60 loading and require spacing of cars to avoid having cars hauling maximum loads adjacent to each other. The initial cost to get this operation up and running would include the Priority 2 and 3 repairs. This railroad will cost approximately \$750,000 per year to maintain and another \$1,750,000 per year for upgrades changing out structures on some sort of priority basis (probably matched to speed). This means current repairs are done to maintain the E-60 standards but portions of the Line would be upgraded to E-80 and higher speeds over a long period of time. This method of railroad operations will slowly begin reversing the deferred maintenance of the bridge system.

A railroad with an immediate need for increased capacity improvements to upgrade much of the bridge system to a higher standard now, meaning we would still operate with E-60 engines but could handle E 72 rail car loads (286,000 pound cars) on the bridges and tunnels while having a lower speed, the average could start around 25 mph. This bridge system would cost approximately \$70,000,000 to get up and running, the big change being heavier rail and ties on bridges. This would require a more detailed study of the bridges currently carrying the lighter loads to be assured that the current steel bridges could handle the higher loads. This bridge system will cost approximately \$500,000 per year to maintain and another \$1,500,000 per year for upgrades for approximately 15-20 years to finish the upgrade cycle.

### **Cost Estimating Methodology**

Opinions for five primary costs (immediate cost to start operation, short-term (within 5 years), long-term (beyond 5 years), rehabilitation, and complete replacement costs) were developed for each bridge. Calculations for each of these costs are contained in Attachment 3 of this report. In addition, the estimated cost to remove all of the bridges has been calculated. These costs have been summarized in Figures No. 1 and No. 2 of this report.

The basis of cost was developed by quantifying the repair recommendations and applying unit costs for each repair. In addition, these costs include an allowance for design and construction engineering. All costs assume a competitive bid for the work by qualified general contractors in the spring of 2009. It should be noted that during repair of items recommended in the report, typically other damaged members that may have been hidden are found, thereby increasing the amount of the work from that shown in the report. Consistent with industry standard, a construction contingency has been added to all costs to account for this unidentified work. All costs in this report are assumed to be 2009 dollars. Future budgetary numbers should be increased for inflation accordingly.

The 2005 and 2007 Osmose Bridge Inspection Reports summarized and prioritized recommended repair work for each bridge. This repair work was quantified from both inspection reports for each bridge. From this work, the Study Team was able to identify the repairs that were recommended in 2007 but not noted for 2005. This difference is assumed to represent the incremental deterioration of the Line over a two year span. Our visual inspections took place during September 2008 and our understanding is the transfer of ownership for the Line is projected for March 2009. Therefore, there will be six months of additional deterioration of the Line beyond the condition observed by our inspectors in September 2008. The Study Team did consider that a lack of traffic on the

## **Coos Bay Rail Link Bridge Condition Assessment Final Report**

Line, from September 2007 to spring 2009, would generally be favorable for decreased deterioration of the bridge system during this time. However, the traffic on the Line from 2005 to 2007 is considered light and the deterioration of materials accelerate as they age. Therefore, these two factors were considered to cancel each other and the base cost of immediate and short-term repairs were increased an amount equal to the difference in recommended repairs and their associated costs from 2005 to 2007 prorated for six months. This increase is intended to account for six months of deterioration of the bridge system in the Pacific Northwest climate from the date of our visual inspection in September 2008 to March of 2009. This increase has been calculated to consist of an additional \$40,775 of Priority 2 repairs, \$81,100 of Priority 3 repairs, and \$442,575 of Priority 4 repairs.

Another consideration is that while the system is not operating, many repairs can be accomplished more efficiently or cost-effectively and there may be some advantages to accomplishing Priority 3 and 4 repairs at this time. Another factor is that aggregating items together to produce large volume contracts can reduce unit prices. For instance, removing a stringer while doing track and tie upgrade projects is much easier and should be accomplished at the same time. These types of considerations have not been included in the Study at this time, as the methodology of the capital improvement program for this Line has not been established.

Cost of bridge system administration is not included in this Study. However, this bridge system should be inspected yearly or biennially. This annual cost (identified by Osmose) is approximately \$50,000.00 dollars per year (2009 cost).

### **Immediate Bridge Costs to Begin Operations**

The basis of the immediate bridge repair costs were developed assuming the Line will have limited freight in the first 5 years with corresponding minimal expenditures for maintenance. The following considerations determined our estimated cost to begin bridge system operations.

- We quantified Priority 2 repairs recommended in the 2007 Osmose reports and applied unit costs for each repair.
- We quantified repairs identified during the 2008 DEA visual inspections and applied unit costs for each repair.
- We have increased the value of the Priority 2 repairs by \$40,775 to reflect additional deterioration of the bridge system, which will occur from September 2008 to March 2009.
- One year cost to reestablish an annual inspection program is \$50,000.
- Our routine inspection of Bridge 716.40 (Siuslaw) recommends a load rating be performed on the steel truss spans of this bridge prior to start of operations. We have assumed this recommendation would hold true for Bridge 739.68 (Umpqua). Cost for each analysis is \$50,000 resulting in a total cost of \$100,000.

Cost calculations for typical and unique repairs by bridge are included in Attachment 3.

The total cost of Osmose and DEA identified Priority 2 repairs is \$9,211,395, coupled with the \$40,775 of additional repairs anticipated for deterioration, which will occur prior to transfer of Line ownership, \$50,000 for inspection, \$100,000 for load ratings of Bridge Numbers 716.4 and 739.68 yields a total bridge system cost to begin operations of \$9,402,170.



## **Coos Bay Rail Link Bridge Condition Assessment Final Report**

### **Anticipated Short-Term Bridge Costs (Years 2, 3, 4, 5)**

The basis of the short-term bridge operating cost was developed by addition of the following considerations

- We assigned a \$1,200,000 (2009 dollars) per year for a bridge maintenance budget assuming the Line would be operated to minimize expenditures
- We allowed for \$50,000 per year inspection budget.

The total short-term (four total years) costs assuming the Line is managed to minimize expenditures is \$5,000,000

Note The short-term bridge costs can be exceeded if Priority 3 and 4 repairs are down-graded to Priority 1 or 2. A conservative upward boundary for short-term costs may be the magnitude of the Priority 3 repairs or \$15,000,000. This amount is approximately equal to the risk of operating the system, while minimizing expenditures.

### **Anticipated Long-Term Bridge Costs (Beyond 5 Years from Start of Operation)**

The basis of the short-term bridge operating cost was developed by addition of the following considerations

- We assigned a \$1,200,000 (2009 dollars) per year for a bridge maintenance budget assuming the Line would be operated to minimize expenditures
- We allowed for \$50,000 per year inspection budget.

The long-term costs, assuming the Line is managed to minimize expenditures, is \$1,250,000 per year (2009 dollars).

Note The long-term bridge costs can be exceeded if Priority 3 and 4 repairs are down-graded to Priority 1 or 2. A conservative upward boundary for long-term costs may be the magnitude of the Priority 3 and 4 repairs or \$30,000,000. This amount is approximately equal to the risk of operating the system, while minimizing expenditures.

### **Rehabilitation of Bridge System**

Another reasonable assumption for the long-term bridge costs for a bridge system of this age could be assumed to be the cost to rehabilitate and upgrade the entire bridge system with an increase in capacity and service life. This cost is estimated to be approximately \$119,000,000. This rehabilitation would indefinitely restore the bridge system for the foreseeable future and reduce or eliminate unplanned expenditures on the Line, as well as reduce or eliminate unplanned loss of service resulting from the discovery of new Priority 1 or 2 repairs. These bridge costs are a planning level estimate based upon a competitive bid of the work by qualified contractors. Rehabilitation cost calculations are noted in Attachment 3.

### **Replacement Value of Bridge System**

The cost to construct this bridge system should the Line need to be reestablished in the future is \$400,000,000. This cost is for the bridges only and does not include costs associated with reestablishing the track, ballast, general grading, signals, communication systems, signage, and right of way. This cost is presented in 2009 dollars, which will need to be increased for anticipated year of construction. These costs are a planning level estimate based upon a competitive bid of the work by qualified contractors. In addition, these costs have been increased to provide an allowance for design and construction engineering, as well as a contingency to account for conceptual understanding of the replacement requirements. Replacement calculations are noted in Attachment 3.

### **Removal Cost of Bridge System**

Nearly all materials in the existing bridge system are hazardous waste and consist primarily of creosote treated timbers or have lead based paint on the steel. The cost to remove and dispose of this bridge system (including contractor mobilization and contingencies) is estimated to be \$31,840,725. This cost does not include the cost of permitting the removal of these bridges, which primarily exist in waterways influencing endangered species. As discussed in the Reply Verified Statement of Dana Siegfried of DEA filed by the Port on September 12, because

## **Coos Bay Rail Link Bridge Condition Assessment Final Report**

work will need to occur during the approved in-water work windows for these sites, costs for labor during these times will be at a premium. In addition, our experience relating to the demolition of a large steel truss over an environmentally sensitive waterway results in our opinion that any bridge removal in Oregon will require work to be isolated from waters, i.e. coffer dams, and each truss span will be required to be fully coated (to contain lead), picked up as one piece, moved off site and out of waters or wetlands, and then dismantled and disposed of the lead coated members in a legal manner, followed by removal of the concrete foundations. Our removal costs incorporate these factors but do not include removal costs of several generations of treated timber ties and other materials, which have been routinely discarded along the Line over the years. Removal cost calculations are included as a part of the replacement calculations noted above and are included in Attachment 3.

### **Summary**

Priority 2-3-4, rehabilitation, replacement, and removal costs for the bridge system are summarized by bridge in Figures No. 1 and 2. All costs are calculated for 2009 construction. There is no additional allowance for inflation due to construction in subsequent years.

Rehabilitation expenditures will reduce the Port's exposure to unplanned expenditures, interruptions of service, as well as increase the systems' capacity.

### **Abbreviations used in Figures 1 and 2 below**

AR	Arch	PCI	Prestressed concrete "I" beam
BD	Ballast deck trestle	PCB	Prestressed concrete box
BM	Beam span	PCS	Prestressed concrete slab
CTG	Concrete "T" girder	RCS	Reinforced concrete slab
CB	Concrete box	IB	Steel "I" beam
CBG	Concrete box girder	SBS	Steel beam span
CS	Concrete slab	SPT	Steel pile trestle
DPT	Deck pinned truss	TPCT	Thru pinned connected truss
DPG	Deck plate girder	TPT	Thru pinned truss
DPLG	Deck plate lattice girder	TPG	Thru plate girder
DRT	Deck riveted truss	TRT	Thru riveted truss
FT	Frame trestle-all timber	WFB	Wide flange beam
OD	Open deck	WFBS	Wide flange beam span
PT	Pile trestle-all timber	WSP	Wide steel beam

Coos Bay Rail Link Bridge Cost Summary Figure No. 1											
BRIDGE LOCATION	EXISTING BRIDGE				PRIORITY LEVEL COSTS				OVERALL TOTAL COSTS		
	STRUCTURE DESCRIPTION	YEAR BUILT	SPANS OVER		PRIORITY 2 TOTAL	PRIORITY 3 TOTAL	PRIORITY 4 TOTAL	REHABILITATION TOTAL	REPLACEMENT TOTAL		
652.21	120 ft OD PT	1940	Amazon creek		\$161,760	\$2,400	\$39,200	\$396,000	\$859,815		
652.58	8 ft OD PT		Drainage		\$0	\$12,800	\$0	\$0	\$57,321		
656.12	100 ft OD TRT	1912	Coyote Creek		\$0	\$14,875	\$59,500	\$907,500	\$3,403,125		
657.12	120 ft OD PT	1941	Coyote Creek		\$12,800	\$25,760	\$32,960	\$396,000	\$859,815		
657.96	30 ft BD PT	1927	Coyote Creek		\$0	\$0	\$28,120	\$99,000	\$214,954		
658.17	120 ft OD SPT	1923	Water		\$46,240	\$16,640	\$17,120	\$594,000	\$948,915		
660.89	90 ft OD PT	1943	Drainage		\$12,800	\$16,320	\$26,080	\$297,000	\$644,861		
661.28	90 ft OD PT, 63 ft OD TPG, 90 ft OD PT	1932, 1914, & 1932	Long Tom Creek		\$12,800	\$102,880	\$105,280	\$1,009,800	\$2,448,245		
661.73	135 ft OD PT	1934	Long Tom Overflow		\$0	\$27,200	\$52,640	\$445,500	\$967,292		
662.59	345 ft OD PT	1936	Long Tom Overflow		\$52,000	\$120,320	\$87,360	\$1,138,500	\$2,471,968		
664.00	100 ft OD TRT, 135 ft BD PT	1914 & 1928	Long Tom Creek		\$56,000	\$216,125	\$5,250	\$1,397,550	\$4,616,884		
664.62	90 ft OD PT, 100 ft OD TRT, 90 ft OD PT	1939, 1914, & 1939	Long Tom Creek		\$14,000	\$88,900	\$76,650	\$1,560,900	\$4,971,557		
664.85	75 ft OD PT, 100 ft OD TRT, 75 ft OD PT	1939, 1914, & 1939	Long Tom Creek		\$35,000	\$118,125	\$30,800	\$1,452,000	\$4,735,108		
665.49	105 ft OD PT, 100 ft OD TRT, 90 ft OD PT	1933, 1914, & 1933	Elk Creek		\$15,750	\$45,500	\$23,975	\$1,615,350	\$5,089,782		
666.21	60 ft BD PT	1930	over Rd.		\$1,600	\$0	\$16,000	\$198,000	\$443,891		
667.28	75 ft OD PT	1928	Drainage		\$0	\$22,400	\$20,160	\$247,500	\$537,384		
667.40	90 ft OD PT, 160 ft OD DPG, 45 ft OD PT	1933, 1914, & 1934	Noti Creek and Vaughn Rd		\$427,200	\$107,200	\$70,720	\$1,501,500	\$2,232,512		
668.50	154 ft OD PT, 120 ft OD DPG, 90 ft OD PT	1933, 1944, & 1921	Noti Creek		\$104,160	\$55,040	\$54,880	\$1,587,200	\$2,779,788		
671.89	90 ft OD PT	1943	over RD		\$0	\$29,920	\$67,840	\$297,000	\$665,837		
674.03	30 ft OD PT	1956	Stream		\$22,720	\$320	\$18,560	\$99,000	\$214,954		
676.23	60 ft OD DPG	1913	Chikkahominy Creek		\$32,640	\$11,200	\$0	\$396,000	\$1,054,350		
677.05	60 ft OD SPT	1914	Walker Creek		\$2,400	\$45,120	\$16,320	\$297,000	\$474,458		
677.80	56 ft OD PT, 80 ft OD TPG, 45 ft OD PT	1932, 1914, & 1951	Wild Cat Creek		\$0	\$55,840	\$29,120	\$861,300	\$2,194,818		
678.43	75 ft OD PT, 122 ft OD TPG, 45 ft OD PT	1921, 1914, & 1927	Wild Cat Creek		\$8,000	\$183,600	\$53,280	\$1,201,200	\$3,103,304		
680.17	150 ft OD TRT	1914	Wild Cat Creek		\$0	\$71,050	\$24,500	\$1,361,250	\$5,817,075		
680.46	102 ft OD TPG	1914	Wild Cat Creek		\$80,000	\$326,400	\$2,400	\$673,200	\$2,903,175		
680.77	102 ft OD TPG	1914	Wild Cat Creek		\$40,000	\$42,080	\$0	\$673,200	\$2,903,175		

Coos Bay Rail Link Bridge Cost Summary      Figure No. 1

EXISTING BRIDGE										
BRIDGE LOCATION	STRUCTURE DESCRIPTION	YEAR BUILT	SPANS OVER		PRIORITY LEVEL COSTS				OVERALL TOTAL COSTS	
					PRIORITY 2 TOTAL	PRIORITY 3 TOTAL	PRIORITY 4 TOTAL	REHABILITATION TOTAL	REPLACEMENT TOTAL	
681.05	162 ft OD TPG	1914	Wild Cat Creek		\$83,200	\$6,400	\$82,400	\$1,069,200	\$2,879,059	
681.45	222 ft OD DPG	1914	Wild Cat Creek		\$104,800	\$160,800	\$2,400	\$1,465,200	\$4,082,414	
682.18	200 ft OD TPG, 50 ft OD DPG	1914	Siuslaw River		\$88,000	\$148,800	\$116,000	\$1,650,000	\$6,360,131	
683.36	155 ft OD TPG, 200 ft OD TRT, 120 ft OD TPG	1914	Siuslaw River		\$88,175	\$20,125	\$10,150	\$3,811,500	\$15,945,909	
687.03	65 ft OD PT	1922	Rock Creek		\$0	\$63,680	\$32,320	\$214,500	\$465,733	
687.88	90 ft OD PT	1922	Meadow Creek		\$12,160	\$45,920	\$25,760	\$297,000	\$644,861	
689.23	15 ft BD PT	1926	Creek		\$840	\$8,000	\$3,200	\$0	\$102,713	
690.47	45 ft OD TPG	1914	San Antonio Creek		\$22,560	\$2,400	\$0	\$297,000	\$341,550	
690.85	64 ft OD TPG, 114 ft OD TRT, 108 ft OD TPG	1914	Siuslaw River		\$29,400	\$39,025	\$220,850	\$2,283,270	\$8,124,612	
691.05	70 ft OD DPG, 250 ft OD TRT, 135 ft OD PT	1914 & 1934	Siuslaw River		\$259,875	\$279,125	\$476,000	\$3,267,000	\$11,970,356	
691.38	15 ft OD FT	1914	Creek		\$8,000	\$12,800	\$960	\$0	\$102,713	
692.06	16 ft BD PT		Creek		\$32,000	\$0	\$12,800	\$0	\$109,560	
693.27	75 ft BD PT	1928	Creek		\$640	\$21,760	\$12,960	\$247,500	\$537,384	
694.32	150 ft OD PT, 200 ft OD TPT, 90 ft OD PT	1936, 1914, & 1925	Siuslaw River		\$458,850	\$344,050	\$126,175	\$2,686,200	\$9,886,668	
694.78	90 ft OD DPG, 150 ft OD TRT, 90 ft OD DPG	1914	Siuslaw River		\$216,300	\$304,938	\$810,250	\$2,668,050	\$9,296,430	
694.98	15 ft OD FT	1914	Creek		\$0	\$20,160	\$0	\$0	\$102,713	
696.66	75 ft BD FT, 150 ft OD TRT, 135 ft OD PT	1914, 1914, & 1921	Lake Creek		\$3,325	\$62,300	\$154,175	\$2,123,550	\$7,696,825	
700.37	120 ft OD PT	1939	Thompson Creek		\$10,400	\$76,480	\$26,560	\$396,000	\$859,815	
702.65	135 ft OD PT	1938	Creek		\$37,600	\$13,600	\$17,280	\$445,500	\$967,292	
702.95	60 ft OD PT	1946	Creek		\$22,720	\$22,400	\$0	\$198,000	\$429,908	
710.20	105 ft BD PT	1925	Olsen Creek		\$35,520	\$14,400	\$30,880	\$346,500	\$752,338	
711.37	75 ft BD PT	1928	Creek		\$19,040	\$800	\$30,080	\$247,500	\$537,384	
711.67	75 ft BD PT	1931	Creek		\$24,000	\$27,200	\$34,400	\$247,500	\$537,384	
712.16	60 ft BD PT	1926	Creek		\$8,000	\$20,160	\$42,720	\$198,000	\$429,908	
712.80	60 ft OD PT	1927	Creek and Access Rd		\$34,400	\$20,160	\$19,200	\$198,000	\$429,908	
716.40	4 ft OD TPG, 2670 ft OD PT, 400 ft OD TPT, 284 ft OD TR	1933, 1933, 1915, & 1931	Siuslaw river		\$2,462,040	\$2,408,175	\$884,450	\$16,327,740	\$69,877,691	
718.12	30 ft BD CB	1975	slough		\$8,000	\$0	\$12,800	\$0	\$0	
719.07	105 ft OD PT	1932	Creek and Private Access		\$64,320	\$63,360	\$26,560	\$346,500	\$752,338	
719.21	315 ft OD PT	1939	Private Access and Marsh		\$154,240	\$117,760	\$98,240	\$1,039,500	\$2,257,014	
724.22	120 ft BD CB	1970	Maple Creek		\$8,000	\$12,800	\$0	\$0	\$0	
725.96	646 ft OD PT	1975	Lake Siltcoos		\$61,120	\$29,440	\$17,600	\$2,131,800	\$4,628,671	

Coos Bay Rail Link Bridge Cost Summary      Figure No. 1

BRIDGE LOCATION	EXISTING BRIDGE				PRIORITY LEVEL COSTS				OVERALL TOTAL COSTS	
	STRUCTURE DESCRIPTION	YEAR BUILT	SPANS OVER		PRIORITY 2 TOTAL	PRIORITY 3 TOTAL	PRIORITY 4 TOTAL	REHABILITATION TOTAL	REPLACEMENT TOTAL	
726.31	990 ft OD PT	1973	Lake Siltcoos	\$481,440	\$125,120	\$260,800	\$3,267,000	\$7,093,474		
727.35	1200 ft BD CB	1977	Lake Siltcoos	\$7,040	\$800	\$0	\$0	\$0		
728.51	270 ft OD PT	1930	Lake Siltcoos	\$96,320	\$177,280	\$57,120	\$891,000	\$1,934,584		
729.04	390 ft OD PT	1932	Wetland	\$198,080	\$132,320	\$166,560	\$1,287,000	\$2,794,399		
729.17	480 ft BD CB	1981	Lake Tahkenitch	\$7,360	\$12,800	\$0	\$0	\$0		
730.56	990 ft OD PT, 120 ft OD CB, 102 ft OD DPG	1953, 1915, & Steel 1915	Lake Tahkenitch	\$96,000	\$449,920	\$376,960	\$4,534,200	\$17,798,839		
731.65	945 ft OD PT, 102 ft OD DPG, 1245 ft OD PT	1960, 1918, & 1915	Lake Tahkenitch	\$49,280	\$1,082,560	\$776,000	\$7,900,200	\$17,567,327		
732.84	375 ft OD PT, 28 ft OD DPG, 228 ft OD PT	1915, 1915, & 1944	Lake Tahkenitch	\$30,720	\$252,960	\$219,520	\$2,174,700	\$4,541,984		
733.88	1120 ft BD CB	1980	Lake Tahkenitch	\$0	\$12,800	\$0	\$0	\$0		
733.95	1178 ft BD CB	1980	Lake Tahkentitch	\$6,400	\$21,200	\$0	\$0	\$0		
735.86	80 ft OD PT	1931	Wetland	\$57,920	\$27,200	\$49,280	\$264,000	\$573,210		
736.03	50 ft OD PT	1926	Jack Frank creek	\$27,520	\$41,600	\$17,600	\$165,000	\$358,256		
736.51	48 ft OD PT	1926	Grade Separation	\$0	\$28,160	\$39,520	\$158,400	\$314,820		
737.33	80 ft BD PT	1931	Jack Franz Creek	\$0	\$0	\$43,040	\$297,000	\$644,861		
738.70	30 ft OD PT	1927	Grade Separation	\$58,400	\$36,640	\$32,960	\$99,000	\$196,763		
738.94	405 ft OD PT	1915	Smith River slough	\$3,840	\$97,440	\$83,200	\$1,336,500	\$2,901,876		
739.14	375 ft OD PT, 50 ft OD TPG, 165 ft OD PT	1937, 1915, & 1915	Smith River slough	\$154,400	\$396,960	\$132,800	\$2,112,000	\$4,284,549		
739.43	288 ft OD PT, 92 ft OD TPG, 119 ft OD PT	1915	Smith River slough	\$61,120	\$145,600	\$141,600	\$1,950,300	\$4,608,017		
739.68	80 ft OD PT, 1125 ft OD TPT, 360 ft OD TPT, 60 ft OD PT	1915	Umpqua River	\$1,698,200	\$4,922,750	\$5,524,400	\$13,984,575	\$85,714,419		
740.25	60 ft OD PT	1914	over Rd	\$16,000	\$3,200	\$31,200	\$198,000	\$443,891		
740.84	60 ft OD PT	1980	Scofield creek	\$2,400	\$12,800	\$0	\$198,000	\$443,891		
741.35	60 ft OD PT	1915	Creek	\$7,360	\$34,720	\$33,760	\$198,000	\$429,908		
741.74	45 ft OD PT	1915	Creek	\$23,680	\$37,120	\$31,360	\$148,500	\$322,431		
742.05	45 ft OD PT	1915	Creek - Wetland	\$0	\$65,120	\$8,000	\$148,500	\$322,431		
742.24	120 ft OD PT	1928	Creek - Wetland	\$0	\$14,400	\$64,960	\$396,000	\$859,815		
742.72	135 ft OD PT	1915	Scofield creek	\$0	\$21,120	\$89,120	\$445,500	\$967,292		
742.95	60 ft OD PT	1936	Creek	\$0	\$65,760	\$18,560	\$198,000	\$429,908		
743.20	45 ft BD PT	1929	Grade Separation	\$0	\$36,160	\$10,400	\$148,500	\$295,144		
743.73	135 ft OD PT	1934	Scofield creek	\$238,400	\$54,560	\$50,240	\$445,500	\$967,292		
743.88	120 ft OD PT	1934	Scofield creek	\$12,800	\$91,200	\$42,080	\$396,000	\$859,815		
743.97	105 ft OD PT	1932	Scofield creek	\$8,240	\$94,720	\$77,760	\$346,500	\$752,338		

Coos Bay Rail Link Bridge Cost Summary      Figure No. 1

Coos Bay Rail Link Bridge Cost Summary									
EXISTING BRIDGE									
BRIDGE LOCATION	STRUCTURE DESCRIPTION	YEAR BUILT	SPANS OVER	PRIORITY LEVEL COSTS			OVERALL TOTAL COSTS		
				PRIORITY 2 TOTAL	PRIORITY 3 TOTAL	PRIORITY 4 TOTAL	REHABILITATION TOTAL	REPLACEMENT TOTAL	
744.24	75 ft BD PT	1929	Wind creek	\$400	\$10,560	\$24,800	\$247,500	\$537,384	
744.44	75 ft BD PT	1929	Wind creek	\$0	\$7,200	\$3,200	\$247,500	\$537,384	
744.70	75 ft BD PT	1929	Wind creek	\$0	\$3,200	\$25,760	\$247,500	\$537,384	
744.83	75 ft BD PT	1929	Wind creek	\$0	\$0	\$13,600	\$247,500	\$537,384	
748.06	90 ft BD PT	1931	Small Creek	\$14,720	\$0	\$0	\$297,000	\$644,861	
748.44	30 ft BD PT	1929	Small Creek	\$0	\$0	\$24,800	\$99,000	\$214,954	
748.68	75 ft OD FT	1915	Small Creek	\$89,760	\$26,400	\$39,680	\$247,500	\$537,384	
749.89	930 ft BD CB	1978	Black Lake	\$7,040	\$12,800	\$0	\$0	\$0	
750.46	960 ft OD PT, 60 ft OD SBS, 513 ft OD PT	1930, 1915, & 1930	North Lake	\$29,600	\$139,200	\$8,160	\$5,256,900	\$11,657,584	
751.02	320 ft OD PT	1915	North Lake	\$32,000	\$25,600	\$28,320	\$1,056,000	\$2,292,840	
752.99	180 ft OD PT	1932	Ten Mile creek	\$0	\$16,000	\$35,200	\$594,000	\$1,289,723	
753.48	105 ft OD PT	1932	Grade Separation	\$12,800	\$5,440	\$20,400	\$346,500	\$688,669	
753.97	41 ft OD SBS	1929	over hwy	\$800	\$0	\$10,400	\$270,600	\$311,782	
755.63	75 ft OD PT	1940	Clear Lake	\$8,160	\$12,800	\$33,920	\$247,500	\$537,384	
756.13	75 ft OD FT	1915	Saunders Lake	\$20,000	\$16,000	\$3,200	\$247,500	\$537,384	
756.55	75 ft OD PT		Saunders Lake	\$64,000	\$59,200	\$42,240	\$247,500	\$537,384	
757.37	75 ft OD PT	1915	Butterfield Lake	\$57,120	\$13,600	\$20,160	\$247,500	\$537,384	
761.13	45 ft OD SPT	1912	Tidal Creek	\$26,880	\$61,760	\$8,160	\$222,750	\$355,843	
PRIORITY SUBTOTAL				\$9,241,395	\$15,039,783	\$13,007,685	\$118,973,085	\$399,517,887	
Estimated Increased Repair Costs for 6 Months of Additional Deterioration to Projected Ownership Transfer March 2008				\$40,775	\$81,100	\$442,575			
				\$9,282,170	\$15,120,883	\$13,450,260			
TOTAL COST									

Coos Bay Rail Link Bridge Condition Assessment  
Final Report

Coos Bay Rail Link Bridge Removal Cost Summary Figure No. 2					
BRIDGE LOCATION	EXISTING BRIDGE		YEAR BUILT	OVERALL REMOVAL COSTS	
	STRUCTURE DESCRIPTION	SPANS OVER		TOTAL	
652 21	120 ft OD PT	Amazon creek	1940	\$36,000	
652 56	8 ft OD PT	Drainage		\$2,400	
656 12	100 ft OD TRT	Coyote Creek	1912	\$125,000	
657 12	120 ft OD PT	Coyote Creek	1941	\$36,000	
657 96	30 ft BD PT	Coyote Creek	1927	\$9,000	
658 17	120 ft OD SPT	Water	1923	\$90,000	
660 99	90 ft OD PT	Drainage	1943	\$27,000	
661 26	90 ft OD PT, 63 ft OD TPG, 90 ft OD PT	Long Tom Creek	1932, 1914, & 1932	\$101,250	
661 73	135 ft OD PT	Long Tom Overflow	1934	\$40,500	
662 59	345 ft OD PT	Long Tom Overflow	1936	\$103,500	
664 00	100 ft OD TRT, 135 ft BD PT	Long Tom Creek	1914 & 1926	\$165,500	
664 62	90 ft OD PT, 100 ft OD TRT, 90 ft OD PT	Long Tom Creek	1939, 1914, & 1939	\$179,000	
664 65	75 ft OD PT, 100 ft OD TRT, 75 ft OD PT	Long Tom Creek	1939, 1914, & 1939	\$170,000	
665 49	105 ft OD PT, 100 ft OD TRT, 90 ft OD PT	Elk Creek	1933, 1914, & 1933	\$183,500	
666 21	60 ft BD PT	over Rd	1930	\$37,500	
667 26	75 ft OD PT	Drainage	1926	\$22,500	
667 40	90 ft OD PT, 160 ft OD DPG, 45 ft OD PT	Not Creek and Vaughn Rd	1933, 1914, & 1934	\$160,500	
668 50	154 ft OD PT, 120 ft OD DPG, 90 ft OD PT	Not Creek	1933, 1944, & 1921	\$213,250	
671 89	90 ft OD PT	over RD	1943	\$56,250	
674 03	30 ft OD PT	Stream	1856	\$9,000	
676 23	60 ft OD DPG	Chickahominy Creek	1913	\$45,000	
677 05	60 ft OD SPT	Walker Creek	1914	\$45,000	
677 80	56 ft OD PT, 80 ft OD TPG, 45 ft OD PT	Wild Cat Creek	1932, 1914, & 1951	\$90,300	
678 43	75 ft OD PT, 122 ft OD TPG, 45 ft OD PT	Wild Cat Creek	1921, 1914, & 1927	\$127,500	
680 17	150 ft OD TRT	Wild Cat Creek	1914	\$630,000	
680 46	102 ft OD TPG	Wild Cat Creek	1914	\$76,500	
680 77	102 ft OD TPG	Wild Cat Creek	1914	\$76,500	

Coos Bay Rail Link Bridge Condition Assessment  
Final Report

Coos Bay Rail Link Bridge Removal Cost Summary Figure No. 2					
BRIDGE LOCATION	EXISTING BRIDGE		YEAR BUILT	SPANS OVER	OVERALL REMOVAL COSTS
	STRUCTURE DESCRIPTION				
681 05	162 R OD TPG		1914	Wild Cat Creek	\$121,500
681 45	222 R OD DPG		1914	Wild Cat Creek	\$188,500
682 18	200 R OD TPG, 50 R OD DPG		1914	Suslaw River	\$187,500
683 36	155 R OD TPG, 200 R OD TRT, 120 R OD TPG		1914	Suslaw River	\$458,250
687 03	65 R OD PT		1922	Rock Creek	\$19,500
687 86	90 R OD PT		1922	Meadow Creek	\$27,000
689 23	15 R BD PT		1926	Creek	\$4,500
690 47	45 R OD TPG		1914	San Antonio Creek	\$33,750
690 65	64 R OD TPG, 114 R OD TRT, 108 R OD TPG		1914	Suslaw River	\$271,500
691 05	70 R OD DPG, 250 R OD TRT, 135 R OD PT		1914 & 1934	Suslaw River	\$405,500
691 36	15 R OD FT		1914	Creek	\$4,500
692 06	16 R BD PT			Creek	\$4,800
693 27	75 R BD PT		1926	Creek	\$22,500
694 32	150 R OD PT, 200 R OD TPT, 90 R OD PT		1936, 1914, & 1925	Suslaw River	\$912,000
694 78	90 R OD DPG, 150 R OD TRT, 90 R OD DPG		1914	Suslaw River	\$765,000
694 96	15 R OD FT		1914	Creek	\$4,500
698 66	75 R BD FT, 150 R OD TRT, 135 R OD PT		1914, 1914, & 1921	Lake Creek	\$693,000
700 37	120 R OD PT		1939	Thompson Creek	\$36,000
702 65	135 R OD PT		1938	Creek	\$40,500
702 85	60 R OD PT		1946	Creek	\$18,000
710 20	105 R BD PT		1925	Olean Creek	\$31,500
711 37	75 R BD PT		1928	Creek	\$22,500
711 67	75 R BD PT		1931	Creek	\$22,500
712 16	60 R BD PT		1928	Creek	\$18,000
712 80	60 R OD PT		1927	Creek and Access Rd	\$18,000
716 40	54 R OD TPG, 2670 R OD PT, 400 R OD TPT, 284 R OD TRT		1933, 1933, 1915, & 1931	Suslaw river	\$3,714,300
718 12	30 R BD CB		1975	slough	\$0
719 07	105 R OD PT		1932	Creek and Private Access	\$31,500
719 21	315 R OD PT		1939	Private Access and Marsh	\$94,500
724 22	120 R BD CB		1970	Maple Creek	\$0
725 96	646 R OD PT		1975	Lake Silcoos	\$193,800



Coos Bay Rail Link Bridge Condition Assessment  
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Coos Bay Rail Link Bridge Removal Cost Summary Figure No. 2					
BRIDGE LOCATION	EXISTING BRIDGE		YEAR BUILT	SPANS OVER	OVERALL REMOVAL COSTS
	STRUCTURE DESCRIPTION				TOTAL
726 31	990 ft OD PT		1973	Lake Siltcoos	\$297,000
727 35	1200 ft BD CB		1977	Lake Siltcoos	\$0
728 51	270 ft OD PT		1930	Lake Siltcoos	\$81,000
729 04	390 ft OD PT		1932	Wetland	\$117,000
729 17	480 ft BD CB		1981	Lake Tahkenitch	\$0
730 58	990 ft OD PT, 120 ft OD CB, 102 ft OD DPG		1953, 1915, & Steel 1915	Lake Tahkenitch	\$408,500
731 65	945 ft OD PT, 102 ft OD DPG, 1245 ft OD PT		1980, 1918, & 1915	Lake Tahkenitch	\$733,500
732 84	375 ft OD PT, 28 ft OD DPG, 228 ft OD PT		1915, 1915, & 1944	Lake Tahkenitch	\$201,900
733 88	1120 ft BD CB		1980	Lake Tahkenitch	\$0
733 95	1178 ft BD CB		1980	Lake Tahkenitch	\$0
735 86	80 ft OD PT		1931	Wetland	\$24,000
736 03	50 ft OD PT		1926	Jack Frank creek	\$15,000
736 51	48 ft OD PT		1926	Grade Separation	\$14,400
737 33	80 ft BD PT		1931	Jack Franz Creek	\$27,000
738 70	30 ft OD PT		1927	Grade Separation	\$9,000
738 94	405 ft OD PT		1915	Smith River slough	\$121,500
739 14	375 ft OD PT, 50 ft OD TPG, 165 ft OD PT		1937, 1915, & 1915	Smith River slough	\$199,500
739 43	288 ft OD PT, 92 ft OD TPG, 119 ft OD PT		1915	Smith River slough	\$191,100
739 68	80 ft OD PT, 1125 ft OD TPT, 360 ft OD TPT, 60 ft OD PT		1915	Umpqua River	\$6,279,000
740 25	60 ft OD PT		1914	over Rd	\$37,500
740 84	60 ft OD PT		1980	Scofield creek	\$37,500
741 35	60 ft OD PT		1915	Creek	\$18,000
741 74	45 ft OD PT		1915	Creek	\$13,500
742 05	45 ft OD PT		1915	Creek - Wetland	\$13,500
742 24	120 ft OD PT		1928	Creek - Wetland	\$36,000
742 72	135 ft OD PT		1915	Scofield creek	\$40,500
742 95	60 ft OD PT		1936	Creek	\$18,000
743 20	45 ft BD PT		1929	Grade Separation	\$13,500
743 73	135 ft OD PT		1934	Scofield creek	\$40,500
743 88	120 ft OD PT		1934	Scofield creek	\$36,000
743 97	105 ft OD PT		1932	Scofield creek	\$31,500

Coos Bay Rail Link Bridge Condition Assessment  
Final Report

Coos Bay Rail Link Bridge Removal Cost Summary Figure No. 2					OVERALL REMOVAL COSTS
BRIDGE LOCATION	EXISTING BRIDGE		YEAR BUILT	SPANS OVER	TOTAL
	STRUCTURE DESCRIPTION				
744 24	75 R BD PT		1929	Wind creek	\$22,500
744 44	75 R BD PT		1929	Wind creek	\$22,500
744 70	75 R BD PT		1929	Wind creek	\$22,500
744 83	75 R BD PT		1929	Wind creek	\$22,500
748 06	80 R BD PT		1931	Small Creek	\$27,000
748 44	30 R BD PT		1929	Small Creek	\$9,000
748 68	75 R OD FT		1915	Small Creek	\$22,500
749 89	830 R BD CB		1978	Black Lake	\$0
750 48	860 R OD PT, 60 R OD SBS, 513 R OD PT		1830, 1915, & 1930	North Lake	\$488,900
751 02	320 R OD PT		1915	North Lake	\$98,000
752 89	180 R OD PT		1932	Ten Mile creek	\$54,000
753 48	105 R OD PT		1932	Grade Separation	\$31,500
753 97	41 R OD SBS		1929	over hwy	\$30,750
755 83	75 R OD PT		1940	Clear Lake	\$22,500
756 13	75 R OD FT		1915	Saunders Lake	\$22,500
756 55	75 R OD PT			Saunders Lake	\$22,500
757 37	75 R OD PT		1915	Butterfield Lake	\$22,500
761 13	45 R OD SPT		1912	Total Creek	\$33,750
OVER WATER					\$20,959,250
OVER ROADWAY					\$199,500
OVER LAND					\$68,400
SUBTOTAL					\$21,227,150
10% MOBILIZATION					\$2,122,715
40% CONSTRUCTION CONTINGENCIES					\$8,490,860
TOTAL COST					\$31,840,725

**BEFORE THE  
SURFACE TRANSPORTATION BOARD**

**STB FINANCE DOCKET NO. 35160**

**OREGON INTERNATIONAL PORT OF COOS BAY  
—FEEDER LINE APPLICATION—  
COOS BAY LINE  
OF THE CENTRAL OREGON & PACIFIC RAILROAD, INC.**



**SUPPLEMENTAL REPLY OF THE  
OREGON INTERNATIONAL PORT OF COOS BAY**

**Exhibit 4**

**Tunnel Report**

**ENTERED  
Office of Proceedings  
OCT - 1 2008  
Part of  
Public Record**

September 26, 2008

Ms. Betsy Imholt, ODOT Rail Study Director  
ODOT Rail Division  
555 13<sup>th</sup> Street NE, Suite 3  
Salem, OR 9731-4179

**RE: ODOT RAIL STUDY – CENTRAL OREGON AND PACIFIC RAILROAD,  
COOS BAY SUBDIVISION**

*Dear Ms. Imholt:*

As per your e-mailed approval and direction on September 9, 2008, we have coordinated with Rail America and the Central Oregon and Pacific Railroad (CORP) to visit a limit of four of the nine tunnels on the Coos Bay Subdivision. The Coos Bay Alignment and tunnels are shown in Figure 1, and more detailed locations of each of the nine tunnels along the alignment are shown in Figures 2 through 8. This letter documents our assessment of changes that have occurred in Tunnel Nos. 13, 15, 17, and 18 since we prepared an earlier report to Rail America dated July 16, 2007, entitled, "Tunnel Inventory – Coos Bay Subdivision, Oregon," that they subsequently made public. We have also included and updated Tables 1 through 11 to reflect increases in estimated costs to reflect cost escalations since our report of 2007.

As per your authorization, we performed a field review and updated our condition assessments for Tunnel Nos. 13, 15, 17, and 18 on September 12 and 13. CORP staff provided flagging services and designated a railroad employee to escort and provide access via hy-rail to the Shannon & Wilson, Inc. field crew during the tunnel visits. We visited and logged Tunnels 13 and 15 on September 12 and Tunnels 17 and 18 on September 13. The portals to all four tunnels were blocked with locked chain link fence panels; however, in past months vandals had detached the gates in order to run vehicles, possibly recreational all-terrain vehicles (ATVs), through Tunnel 13.

The tunnel review process was undertaken by our project manager, Robert Robinson, and senior engineering geologist, Klaus Winkler, both of whom had previously visited all nine tunnels in order to develop the 2007 tunnel condition assessment report for Rail America.

During our review process, we noted minor changes in the nature and condition of the tunnel support system and the condition and stability of the rock, where visible.

As stated and described in detail in our tunnel inventory report dated July 2007, we identified and classified numerous sections in the tunnels that are in various states of deterioration and, in our opinion, require immediate (within six months) to long-term (30 to 48 months) rehabilitation work in order to reduce the currently high risk of rock falls and timber collapses to more acceptable levels.

Since November 2006, minor rock falls and isolated failed timber sets were observed in tunnels in the Coos Bay Subdivision:

- ▶ Tunnel 13
  - At Station 14+00, five timber ribs that were previously logged as rotted and settled by 6 inches have settled an additional 6 inches and kicked inward by approximately 6 inches.
  - Approximately 5 to 10 percent of the wood foot-blocks have rotated due to ATV traffic in the drainage ditches.
  - Several small 1 cubic foot to 1 cubic yard (cy) rock and wood debris falls.
- ▶ Tunnel 15
  - Station 15+40 west post shifted off foot-block.
  - Station 19+80 rockfall through hole in shotcrete arch of approximately 1 cy.
- ▶ Tunnel 17
  - No significant changes in last year.
- ▶ Tunnel 18
  - No significant changes in last year.

In our opinion, the repairs recommended for tunnel sections that were classified as Repair Level 1 and 2 in our July 2007 report are necessary to reinstate relatively safe train passage. The risk of future rockfalls and failing timber sets is high under the current condition of the tunnels. However, the increased seepage rate in some areas of the tunnels that normally accompanies the rainy season will contribute to an increased risk of instability and also makes the application of

Ms. Betsy Imholt  
ODOT Rail Division  
September 26, 2008  
Page 3

SHANNON & WILSON, INC.

remedial shotcrete in these seepage areas impossible and hazardous. Consequently, it may not be safe for much of the repair work to be undertaken until the drier months of spring and summer.

We appreciate the opportunity to work with you and look forward to answering any questions you have about the information in this report.

Sincerely,

SHANNON & WILSON, INC.



Robert A. Robinson  
Senior Vice President  
Director of Underground Services

RAR/rar

Enclosures: Table 1 - List of Tunnels - Coos Bay Subdivision  
Table 2 - Tunnel 13  
Table 3 - Tunnel 14  
Table 4 - Tunnel 15  
Table 5 - Tunnel 16  
Table 6 - Tunnel 17  
Table 7 - Tunnel 18  
Table 8 - Tunnel 19  
Table 9 - Tunnel 20  
Table 10 - Tunnel 21  
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# **ODOT RAIL STUDY** **LIST OF TUNNELS - Coos Bay Subdivision**

RAILROAD	TUNNEL NO.	MILEPOST @ N PORTAL	LINE	LAT.	LONG.	Tunnel Liner	Condition	TOTAL LENGTH	LENGTH OF TANGENT	LENGTH OF CURVE	DEGREE OF CURVE	Comments
CORP	13	689.47	Coos Bay	44 0279	123 4849	Timber sets, shotcreted steel sets, steel sets with timber lagging, 100 ft unlined, 55 ft concrete portal barrels	Poor - Good	2,486 ft	South 1732	North 755	8°	Rotting timber sets and lagging
CORP	14	681.09	Coos Bay	43 9988	123 6376	Shotcreted steel sets, shotcreted bedrock, 50 ft N portal concrete barrel	Good	471 ft	0	471	8°	Thin shotcrete, cracking and spalling
CORP	15	720.73	Coos Bay	43 9281	124 0408	Shotcrete over steel and timber sets, timber sets and lagging, N & S portal are 50 ft long concrete barrels	Poor - Good	2,143 ft	South 2086	North 57	4°	Rotting timber sets and lagging, several cave-ins, recent repair work
CORP	16	721.52	Coos Bay	43 925	124 0288	Shotcrete over timber and steel sets, N & S portal are 50 ft long concrete barrels	Good	633 ft	0	617	7°	
CORP	17	727.7	Coos Bay	43 8516	124 0858	Shotcrete over bedrock, timber sets, shotcreted steel sets N & S, portal concrete barrels	Poor - Good	1,200 ft	North 958	South 242	2°	Timber sets with rotting footblocks, offsets, cracks in shotcrete
CORP	18	734.48	Coos Bay	43 763	124 1177	Shotcrete over steel and timber sets, timber sets and lagging N & S portal are 50 ft long concrete barrels	Poor - Good	1,580 ft	1550	0	0°	Timber sets with rotting footblocks, offsets cracks in shotcrete
CORP	19	745.62	Coos Bay	43 8473	124 1006	Shotcrete over bedrock, shotcrete over steel sets S portal 50 ft concrete barrel	Fair - Good	4,202 ft	Middle 3857	North & South 227	N 6° S 3°	Landslides over both portals, shotcrete thin and cracking bedrock exposed
CORP	20	750.1	Coos Bay	43 5968	124 1493	Shotcrete over bedrock, shotcrete over steel sets, S portal 50 ft concrete barrel	Fair	874 ft	0	870	2°	Large areas of spalling shotcrete with exposed bedrock
CORP	21	751.2	Coos Bay	43 5831	124 1585	Shotcrete over bedrock, N & S portal are 50 ft long concrete barrels	Good	478 ft	0	475	4°	
Note Only Tunnels 13, 15, 17, and 18 were revisited for this review								Total Length	14,077 ft			

## MP 669.47 to 669.94

[illegible]



ODOT RAIL STUDY  
TUNNEL 13  
Coos Bay Subdivision, Oregon  
MP 669.47 to 669.94

Station		Length, ft	Repair Level	Lining		Concrete Curb		Comments	Repairs		Steel Deck		Reinforcing		Shotcrete		Concrete		Timber Sills		Comments
From	To			Type	Set Spacing, ft	V/N	Above TOR, in				R	No	Reinforcing	UF	ft <sup>2</sup>	cy	ft <sup>2</sup>	cy	R	No	
14+83	14+87	4	2	Unlined Timber Sills on Wood Foot Blocks with Timber Lagging		N		Exposed bedrock. Separated rock blocks in crown, obvious rock-fall hazard. Timber sills at 4'-spacing on wood foot blocks with timber lagging. Advanced deterioration of wood foot blocks; rotting lagging is falling out. - 4'-8"-sag along east sidewalk from Sta 14+80 to 15+12	- Install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row) and apply 6"-thick steel fiber reinforced shotcrete				1	60	288	5					2' block of rock on 14' each
14+87	15+12	25	2, 3		4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Rotting and deteriorated timber sills (3) advanced deterioration of wood foot blocks rotting lagging is falling out. Exposed bedrock along east sidewalk - 2'-3"-sag along west sidewalk from Sta 15+00 to 15+33 (extends into following section - see below)	- Remove timber lining, install rockbolts (row-spacing 5 feet, six 15'-long rockbolts per row), and apply 6"-thick steel fiber reinforced shotcrete			5	360	1800	33			25		7	
15+12	15+20	8	2	Timber Sills on Wood Foot Blocks with Timber Lagging	4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Advanced deterioration of wood foot blocks; rotting lagging. Bedrock appears to be competent	- Remove timber lining, install rockbolts (row-spacing 5 feet, five 15'-long rockbolts per row) and apply 6"-thick steel fiber reinforced shotcrete			2	180	576	10			8		3	
15+20	15+48	28	2	Timber Sills on Wood Foot Blocks with Timber Lagging	4	N		- 7'-sag from Sta 15+29 to 15+33 and from Sta 15+33 to 15+48 along east sidewalk, - 4'-sag from Sta 15+33 to 15+53 along west sidewalk (extends into following section - see below)				6	450	2088	38			29		8	
15+48	15+69	20	3 - 4	Timber Sills on Wood Foot Blocks with Timber Lagging	4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Rotting lagging. Bedrock appears to be competent.	- Remove timber lining, install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete			4	288	1440	18			20		6	
15+69	16+14	45	2	Timber Sills on Wood Foot Blocks with Timber Lagging	4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Deteriorated sills, lagging, and wood foot blocks at various stages	- Remove timber lining, install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete			6	648	3240	40			45		12	
16+14	16+58	44	3 - 4	Timber Sills on Wood Foot Blocks with Timber Lagging	4	N		- Timber lagging in 1/4'-arch rotted - but mostly still in place - along west sidewalk between Sta 16+34 and 16+48 deterioration along backside of timber posts, foot blocks in fair condition				9	648	3168	39			44		12	
16+58	16+89	31	3	Timber Sills on Wood Foot Blocks	4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Rotting lagging condition of foot blocks fair to very poor throughout section	- Remove timber lining, install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete			6	432	2232	28			31		9	
16+89	17+08	18		Steel Sills on I-beams and Timber Sills with Timber Lagging	4	N		- Steel sills installed between existing timber sills (timber sills at 4'-spacing); wood boards were used to strap section together. Steel feet footings are cut I-beams. In-place timber lagging generally dry	- Remove timber ends, wood foot blocks, and timber lagging, clean footings, and apply 4"-thick steel fiber reinforced shotcrete between steel sills. (Use shotcrete to tie in steel web and for footing support)					1368	17						
17+08	18+27	119		Timber Sills with Timber Lagging and Wood Foot Blocks	4	N		Timber sills at 4'-spacing on wood foot blocks with timber lagging. Sills, lagging, and foot blocks generally in fair condition. Apparently competent bedrock close behind lagging typically	- Remove timber lining, install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete			24	1728	8688	107			119		31	
18+27	18+40	13	3	Timber Sills with Timber Lagging on Wood Foot Blocks	4	N		- Damp lagging between Sta 17+20 to 17+36, - Damp and rotted lagging between Sta 17+61 to 17+69, - Drip at east springline at Sta 17+63				3	216	936	11			13		4	-1 cy rock and wood debris in ditch
18+40	18+44	4	2	Unlined				- Drip at west springline at Sta 17+67	- Install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete			1	72	288	3						
18+44	18+20	76		Shotcrete over Steel Sills	2	Y	0	Shotcrete over steel sills at 2'-spacing on concrete curb. Shotcrete application flush with steel sills (~6"-10"-thick). Shotcrete was applied over existing timber lagging	- Backfill void space behind lagging with cementitious material (Potentially 2'-wide void space)												
19+20	19+68	48		Shotcrete over Steel Sills	2	Y	0	Shotcrete over steel sills at 2'-spacing on concrete curb. Shotcrete application ~4'-6"-thick. Shotcrete was applied over existing timber lagging	- Backfill void space behind lagging with cementitious material (Potentially 2'-wide void space)												
19+68	19+75	9	3	Unlined			0	Exposed bedrock in fair condition, RCD ~60-70% in east sidewalk, ~80-90% in crown, ~70-80% in west sidewalk (all east)	- Install rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete			2	144	648	8						-1 cy rock and wood debris in ditch

Table 2

ODOT RAIL STUDY  
TUNNEL 13  
Coos Bay Subdivision, Oregon  
MP 669.47 to 669.94

Shannon Wilson, Inc

Station		Length, ft	Repair Level	Lining		Concrete Curb		Comments	Repairs	Steel Sets		Shotcrete		Concrete		Timber Sets		Comments w/1208 Visit
From	To			Type	Set Spacing, ft	Y/N	Above TOR, in			ft	No	None	LP	ft <sup>2</sup>	cy	ft <sup>2</sup>	cy	
								Steel sets at 2'-spacing on I-beams. Wire mesh was installed between sets. Timber lagging was left in place. Large void space between existing timber lagging and bedrock locally. - Drops from crown between Sta 19+80 to 19+88	(Current conditions of timber lining and wood foot blocks are fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and application of 4'-thick steel fiber reinforced shotcrete. OR Installation of steel channel lagging between steel sets and backfills used along with cementitious material. (Potentially 2'-wide void space)									
19+75	20+25	50		Steel Sets on I-beams	2	N		Shotcreted steel sets at 4'-spacing. Shotcrete application is flush with steel sets. Shotcrete was placed over existing timber lagging. Shotcrete or concrete footing throughout section.										
20+25	21+71	146		Shotcrete over Steel Sets	4	Y		Steel sets at 2'-spacing on I-beams. Existing timber lagging was left in place. Styrodium was used to backfill large void space between lagging and bedrock. - Dip from west springline at Sta 22+57										
21+71	22+97	126		Steel Sets on I-beams	2	N		Steel sets at 4'-spacing on I-beams. Existing timber lagging was left in place. Timber lagging is rolled and falling into tunnel. Rock debris collected behind lagging. Occasional rock fall is evident (see below). - Rockfall material and rolled lagging on tracks along east sidewalk from Sta 22+87 to 23+23	Remove timber lagging and all loose debris behind lagging. - backfill void space with cementitious material. Remove timber lagging and all loose debris behind lagging. - Apply 6" thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									
22+97	23+23	26	2	Steel Sets on I-beams	4	N		Steel sets at 4'-spacing founded on I-beams. Existing timber lagging was left in place. Timber lagging rotted in places	Remove timber lagging and all loose debris behind lagging. - Apply 6"-thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									
23+23	23+39	16	3 - 4	Steel Sets on I-beams	4	N		Steel sets at 4'-spacing founded on I-beams. Existing timber lagging was left in place. Timber lagging is rolled and falling into tunnel. Rock debris collected behind lagging. Rockfall material and rolled lagging on tracks along east sidewalk	Remove timber lagging and all loose debris behind lagging. - Apply 6"-thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									
23+39	23+53	14	2	Steel Sets on I-beams	4	N		Steel sets at 4'-spacing founded on I-beams. Existing timber lagging was left in place. Timber lagging is rolled and ready to fall into tunnel. Backrock appears to be close behind lagging along east sidewalk. Generally 3' to 4' of void space behind lagging in crown. Damp to wet with drips	Remove timber lagging and all loose debris behind lagging. - Apply 6"-thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									
23+53	23+95	42	3 - 4	Steel Sets on I-beams	4	N		Steel sets at 4'-spacing founded on I-beams. Existing timber lagging was left in place. Timber lagging is rolled and ready to fall into tunnel. Backrock appears to be close behind lagging along east sidewalk. Generally 3' to 4' of void space behind lagging in crown. Damp to wet with drips	Remove timber lagging and all loose debris behind lagging. - Apply 6"-thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									
23+95	24+31	36	2	Steel Sets on I-beams	4	N		Steel sets at 4'-spacing founded on I-beams. Existing timber lagging was left in place. Timber lagging is rolled and ready to fall into tunnel. Backrock appears to be close behind lagging along east sidewalk. Generally 3' to 4' of void space behind lagging in crown. Damp to wet with drips	Remove timber lagging and all loose debris behind lagging. - Apply 6"-thick steel fiber reinforced shotcrete between steel sets. Use shotcrete to be in steel web and for footing support. OR Install steel channel lagging between steel sets and backfill void space with cementitious material.									

Table 2

ODOT RAIL STUDY  
TUNNEL 13  
Coos Bay Subdivision, Oregon  
MP 669.47 to 669.94

Shannon Wilson, Inc

Station		Length, ft	Repair Level	Lining		Concrete Curb		Comments	Repairs	Steel Bars		Reinforce	Shrinkage		Concrete		Timber Sills		Comments	
From	To			Type	Set Spacing, ft	Vin	Above TOR, in			R	No		R <sup>2</sup>	CV	R <sup>2</sup>	CV	R	No		
24+31	24+96	65		Concrete Barrel Concrete Portal			Concrete barrel - Crack across barrel at Sta 24+71 with some spalling and water seepage along crack. South Portal @ MP 669.94													
24+96	24+96	0																		
Total Length (ft)		2496																		
Totals										0	0	137	9126	60762	1243	0	0	550	150	

COST ESTIMATE FOR REPAIR LEVELS 1 AND 2 (incl. Level 2-3)

Est. Total Steel Sills (No.)	0 (Est. Unit Rates \$5300/per set)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	2070 (Est. Unit Rates \$85/per LF)	Est. Total Construction Costs	\$175,950
Est. Total Concrete (cy)	0 (Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (cy)	429 (Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$429,000
Est. Total Timber Sills (No.)	38 (Est. Removal Unit Rate \$1800/per set)	Est. Total Removal Costs	\$68,400
Est. Sub Total for Level 1 and 2 (incl. Level 2-3) Repairs			\$685,750

Mobilization (15%) \$99,863  
Contingency (20%) \$133,150  
Est. Total of Level 1 and 2 (incl. Level 2-3) Construction Cost \$888,763

Repair Level

Repairs should be completed immediately to <6 months

Repairs should be completed in 0 to 12 months

Repairs should be completed in 12 - 30 months

Repairs should be completed in 30 - 48 months

No immediate repairs required based on the current conditions

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5

Est. Total Steel Sills (No.)	0 (Est. Unit Rates \$5300/per set)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	9126 (Est. Unit Rates \$85/per LF)	Est. Total Construction Costs	\$776,710
Est. Total Concrete (cy)	0 (Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (cy)	1243 (Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$1,243,000
Est. Total Timber Sills (No.)	150 (Est. Removal Unit Rate \$1800/per set)	Est. Total Removal Costs	\$270,000
Est. Sub Total for Repairs			\$2,289,710

Mobilization (15%) \$338,687  
Contingency (20%) \$451,582  
Est. Total of Construction Cost \$3,048,179

Table 2

**ODOT TUNNEL REVIEW**  
**TUNNEL 14**  
**Coos Bay Subdivision, Oregon**  
**MP 681.09 to 681.18**

Station	To	Length, ft	Repair Level	Limiting	Set Spacing, ft	Concrete Curb	Comments	Repairs	Steel Sds	No.	Reinbars	Lf	#'	cy	#'	cy	Timber Sds	No.	
From	0+00	0		Concrete Portal		N	North Portal @ MAP 681 09 In general dry. Spalling concrete in east sidewalk at Sta. 0+24. Shotcrete cover over bedrock generally in good condition. Shotcrete is relatively thin in crown (<1") and not reinforced.												
0+00	0+50	50		Concrete Barrel		N		Shotcrete cover over bedrock very thin in crown (0.5" to 1") and not reinforced; thickness in sidewalls estimated to be around 2". Large areas with spalling shotcrete in crown. Crown is built up by concrete bedding plane	Cover spalling areas with steel fiber reinforced shotcrete to desired thickness of 4 inches, new application shall extend to the bottom of sidewalls (2'-thick over existing shotcrete)										
0+50	0+90	40		Shotcrete over Bedrock		N													
0+90	1+80	100		Shotcrete over Bedrock		N													
1+80	4+51	281		Shotcrete over Bedrock		N													
4+51	4+71	20		Shotcrete over Steel Sds	2 and 4	Y													
4+71	4+71	0		Shotcrete over Steel Sds	2	Y													
Total Length (ft)		471																	
Repair Level 4-5 (ft)		100																	
Repair Level 5 (ft)		371																	
Total										0	0	0	0	7200	66	0	0	0	0

Est Total Steel Sails (No.)	0	Est Unit Rates \$53000per set	Est Total Construction Costs	\$0
Est Total Rockbolts (LF)	0	Est Unit Rates \$850per LF	Est Total Construction Costs	\$0
Est Total Concrete (cy)	66	Est Unit Rates \$1100per CY	Est Total Construction Costs	\$72,600
Est Total Shotcrete (cy)	66	Est Unit Rates \$1000per CY	Est Total Construction Costs	\$66,000
Est Total Timber Sails (No.)	0	Est Removal Unit Rate \$15000per set	Est Total Removal Costs	\$0

Est Sub Total for Repairs	\$86,000
Mobilization (15%)	\$9,900
Contingency (20%)	\$13,200
Est Total of Construction Cost	\$89,100

Repair Level	Count	Description
Level 1	2	Repairs should be completed immediately to <6 months
Level 2	3	Repairs should be completed in 0 to 12 months
Level 3	3	Repairs should be completed in 12 - 30 months
Level 4	0	Repairs should be completed in 30 - 48 months
Level 5	0	No immediate repairs required based on the current condition

**No immediate repairs required based on the current conditions**

**9/28/2008**

ODOT RAIL STUDY  
TUNNEL 15  
Coos Bay Subdivision, Oregon  
MP 720.73 to 720.14

Shannon Wilson, Inc

Station	From	To	Length, ft	Repair Level	Lining	Type	Set Spacing, ft	Y/N	Concrete Curb	Height above TOR, in	Comments	Repairs	Steel sets	No	Rows	Reinforce	LF	R <sup>2</sup>	cy	Concrete	R <sup>2</sup>	cy	Timber Lags	No	Comments	
00+00 - 04+00	00+00	00+26	0			Concrete Portal																				
04+00 - 00+26	00+26		26			Concrete Barrel																				
00+26 - 01+35	01+35		109			Shotcrete over Steel Sets on Concrete Curb	4.0	Y	6		Steel sets covered with shotcrete, spaced 4 ft, concrete between sets (?)															
01+35 - 01+47	01+47		12			Shotcrete over Steel (and Timber?) Sets on Concrete Curb	N/A	Y	6		Steel (and timber?) sets covered with shotcrete															
01+47 - 02+10	02+10		63			Timber Sets on Concrete Curb with Timber Lagging	1.5	Y	6		Wood ribs spaced 1.5 ft, fair condition, wet, slight deterioration	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, six 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
02+10 - 02+15	02+15		5			Timber Sets on Concrete Curb with Timber Lagging	1.5	Y	6		3 timber sets, offset, cracked and crested joints	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, six 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
02+15 - 02+77	02+77		62			Timber Sets on Concrete Curb with Timber Lagging	1.0-2.0	Y	6		Wood ribs spaced 1-2 ft, fair to good condition, dry	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, six 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
02+77 - 02+83	02+83		16			Timber Sets on Concrete Curb with Timber Lagging	2.0	Y	6		Deteriorated wood sets, wet - heavy dripping	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, ten 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
02+83 - 03+06	03+06		13			Timber Sets on Concrete Curb with Timber Lagging	2.0	Y	6		Timber sets in fair condition, dry	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, ten 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
03+06 - 03+20	03+20		14			Timber Sets on Concrete Curb with Timber Lagging	1.0-2.0	Y	6		Rotting timber sets, up 1-2 ft, sheared, offset joints, wet - a timber set fell into the tunnel on 07/05/2007 at approximately Sta 3+26	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, ten 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
03+20 - 03+32	03+32		12			Timber Sets on Concrete Curb with Timber Lagging	1.0-2.0	Y	6		Rotting timber sets spaced 1.5 ft, dry in general - the east 1/2 inch segment of the timber set located at around Sta 3+43 fell into the tunnel on 01/16/2007	Remove timber sets and lagging and install rockbolts in crown and sidewalls (row-spacing 5 feet, ten 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
03+32 - 03+56	03+56		24			Timber Sets on Concrete Curb with Timber Lagging	1.5	Y	6		Rotting timber sets, spaced 1 ft, heavy dripping	Remove timber sets and lagging and install rockbolts (row-spacing 5 feet, ten to twelve 15-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete. The construction of the new liner may require the use of steel sets for temporary and permanent support also, due to the wet condition of the area backing of the area with concrete or shotcrete maybe required instead of applying shotcrete. Prior to construction, the drilling of probe holes maybe required in order to obtain information about the current general ground condition														
03+56 - 03+72	03+72		16			Timber Sets on Concrete Curb with Timber Lagging	1.0	Y	6		Rotting timber posts along west side, spaced 1.5 ft	Remove timber sets and lagging and install rockbolts (row-spacing 5 feet, six 14-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete														
03+72 - 03+86	03+86		14			Timber Sets on Concrete Curb with Timber Lagging	1.5	Y	6		Wood ribs, fair condition - Timber sets were shotcreted during repairs in November 2006. Rockbolts were installed in crown and sidewalls north of crested infolded area	Section may require more shotcrete between the existing timber sets for completion - or remove current lining and replace with additional rockbolts and a 4" thick application of shotcrete														
03+86 - 04+15	04+15		28			Timber Sets on Concrete Curb with Timber Lagging	1.5-2.0	Y	6		Wood ribs w/ steel straps - 3 full steel sets (4' spacing) were installed after a cave in/collapse occurred in the west sidewall in November 2006, remaining section consist of arch segments only (placed on a well plate supported by in-place timber posts). Low-strength concrete was used to backfill area between and behind steel sets. Rockbolts and/or Spiling were installed through out the section															
04+15 - 04+42	04+42		27			Steel Arches on Steel and Timber Posts, Shotcreted and Backfilled	4.0	Y	6																	

Table 4

ODOT RAIL STUDY  
TUNNEL 15  
Coos Bay Subdivision, Oregon  
MP 720.73 to 720.14

Shannon Wilson, Inc

Station From To	Length, ft	Repair Level	Lining Type	Set Spacing, ft	Y/N	Concrete Curb Height above TOR, in	Comments	Repairs	Steel Sets	Rockbolts	Shotcrete	Concrete	Timber Sets	Comments
									ft	ft	ft <sup>2</sup>	cf	ft	
04+42	04+60	18	Timber Sets on Concrete Curb with Timber Lagging	4 0	Y	6	Wood ribs with longitudinal cracks in several timbers - timber sets were shotcreted during repairs in November 2006. Rockbolts were installed in crown and sidewalls	Section may require more shotcrete between the existing timber sets for compaction or remove current lining and replace with a 4" thick application of shotcrete						
04+60	04+79	19	Timber Sets on Concrete Curb with Timber Lagging	2 0	Y	8	"Tight" timber sets - numerous offsets, poor condition - Timber sets were partially shotcreted in sidewalls during repairs in November 2006. Rockbolts were installed in crown during repairs	Section may require more shotcrete between the existing timber sets for compaction - or remove current lining and replace with additional rockbolts and a 4" thick application of shotcrete						
04+79	04+86	7	Shotcrete over Steel Sets on Concrete Curb	4 0	Y	6	Caved area up ~15 ft (est. 10 to 15 cu yd rock fill material) - Section was stabilized in November 2006 with two steel sets spaced at 4 feet steel channel lagging was installed and arches and crown were backfilled with shotcrete	Remove timber lining install rockbolts (row-spacing 5 feet eight to ten - two rockbolts in each sidewall - 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete						
04+86	05+12	26	Timber Sets on Concrete Curb with Timber Lagging	1 5	Y	6	Rocked timber and crushed butt joints. Boxed sets along east sidewall were reinforced with rockbolts recently	For long-term stability - remove timber lagging behind steel sets and apply 4"-thick shotcrete over exposed ground between sets		5	600	1872	23	
05+12	05+29	17	Steel Sets on Concrete Curb	1 5	Y	6	Steel sets, spaced 1 5 ft				1224	15		
05+29	05+74	45	Timber Sets on Concrete Curb with Timber Lagging	1 5	Y	6	Timber sets, spaced 1 5ft, with polyurethane grout. Sandstone exposed in 1/4 arch	Current condition of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, six 12'-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete						
05+74	05+78	4	Timber Sets on Concrete Curb with Timber Lagging	2 0	Y	6	Rocked timber and crushed butt joints,	Two timber sets were left in place, because they appeared to be tight when attempted to be removed during recent repair works. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete						
05+78	05+85	7	Shotcrete over bedrock		Y	6	Timber sets were removed and replaced with shotcrete and rockbolts	Remove timber lining, install rockbolts (row-spacing 5 feet, ten - two in each sidewall - 12'-long rockbolts per row), and apply 4"-thick steel fiber reinforced shotcrete						
05+85	05+82	7	Shotcrete over bedrock		Y	6	Timber sets were removed and replaced with shotcrete and rockbolts	Current conditions of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete		5	600	1872	23	
05+82	06+18	26	Timber Sets on Concrete Curb with Timber Lagging	1 5-2 0	Y	6	Good wood sets dry, spaced 2 ft						26	18
06+18	08+89	271	Timber Sets on Concrete Curb with Timber Lagging	2 0	Y	6	Steel jump sets, timber sets, wet areas at Sta 12+50 - one split set hanging down in arch	Remove timber sets and timber lagging between steel jump sets, place 4"-thick shotcrete over reposed ground and tie steel sets into shotcrete application						
08+89	09+26	37	Timber Lagging	1 0-1 5	Y	6	Steel sets covered with shotcrete - minor drips, scattered missing shotcrete in arch	Current conditions of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete						
09+26	09+66	62	Shotcrete over Steel Sets on Concrete Curb	2 0	Y	6	Wood ribs sp. 2 ft, good condition dry, sand deposits at toe of posts at Sta 11+25						37	20
09+66	11+10	122	Timber Sets on Concrete Curb with Timber Lagging	2 0	Y	6	Steel ribs spaced 2 ft, covered with shotcrete, dry			24	1728	6784	110	
11+10	12+66	156	Shotcrete over Steel Sets on Concrete Curb	2 0	Y	6							122	62
~10 % of wood blocking blocks are rotted & crushed - worst sets - downgraded to Repair Level 4														

ODOT RAIL STUDY  
 TUNNEL 15  
 Coos Bay Subdivision, Oregon  
 MP 720.73 to 720.14

Shannon Wilson Inc

Station		Length, ft	Repair Level	Lining		Concrete Curb	Comments	Repairs	Steel Sills		Rockbolts	Shotcrete	Concrete		Timber Sills		Comments			
From	To			Type	Set Spacing, ft				V/N	Height above TOR, in			ft	No	ft <sup>2</sup>	cy		ft <sup>2</sup>	cy	ft
							Good wood sills dry spaced 2 ft	Current conditions of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row spacing 5 feet, five 12-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete												
12+68	13+28	62		Timber Sills on Concrete Curb with Timber Laggings Shotcrete over Steel Sills on Concrete Curb	20	Y	Steel ribs spaced 2 ft, covered with shotcrete dry	Current conditions of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete												
13+28	13+80	62			20	Y	Good wood sills dry, spaced 2 ft													
13+90	14+34	44	3	Timber Sills on Concrete Curb with Timber Laggings Steel Sills and Timber Sills on Concrete Curb with Timber Laggings	20	Y	Steel jump sills, poor wood sills, lost 2 sections. Another timber sill fell out on 11/17/2006 at approximately Sta 14+41	Remove timber sills and timber lagging between steel jump sills place 4"-thick shotcrete over exposed ground and tie steel sills and shotcrete application												
14+34	14+47	13			10-15	Y	Good wood sills dry, spaced 2 ft													
14+47	15+05	118		Timber Sills on Concrete Curb with Timber Laggings Shotcrete over Steel (and Timber) Sills	20	Y	Steel- and (possibly) wood ribs, spaced 1 ft, covered with shotcrete	Current conditions of timber lining is fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row spacing 5 feet, five 12-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete			24	1440	8486	106		118				
15+05	16+81	88			10	Y	Steel ribs spaced 2 ft covered with shotcrete local drops scattered missing shotcrete in arch		Repair spalled shotcrete in crown and increase general thickness of shotcrete in crown (around 2 inches)											
16+81	18+95	234			20	Y	Steel ribs spaced 1.5 to 2 ft covered with shotcrete abundant drops, missing shotcrete in arch locally													
18+95	20+60	165		Shotcrete over Steel Sills on Concrete Curb	15 20	Y	Concrete Barrel longitudinal cracks in both sides some displacement along cracks	Support sidewalls of concrete barrel with rockbolts, 1 1/2 rows on each side at a 5' spacing, bolt length estimated to be 12 feet minimum. Initial work may require probe holes to determine thickness of concrete and depth to competent bedrock.												
20+60	21+43	83		Concrete Barrel S Portal		N	South Portal @ MP 721.14													
Total Length (ft.)		2143								20	8	107	9914	53181	624	0	0	368	345.5	
6/12/08 Final																				

Repair Level

Repairs should be completed immediately to <6 months

Repairs should be completed in 0 to 12 months

Repairs should be completed in 12 - 30 months

Repairs should be completed in 30 - 48 months

No immediate repairs required based on the current conditions

COST ESTIMATE FOR REPAIR LEVELS 1 to 2

Est Total Steel Sills (No.)	0	Est Unit Rates \$6300/per set)		Est Total Construction Costs	\$31,600
Est Total Rockbolts (LF)	2800	Est Unit Rates \$85/per LF)		Est Total Construction Costs	\$231,000
Est Total Concrete (cy)	0	Est Unit Rates \$110/per CY)		Est Total Construction Costs	\$0
Est Total Shotcrete (cy)	84	Est Unit Rates \$1000/per CY)		Est Total Construction Costs	\$84,000
Est Total Timber Sills (No.)	72	Est Removal Unit Rates \$1600/per set)		Est Total Removal Costs	\$114,657
Est Sub Total for Level 1 and 2 (and Level 2-3)				Repairs	\$451,487
				Mobilization (15%)	\$67,720
				Contingency (20%)	\$90,283
				Construction Cost	\$609,480

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5

Est Total Steel Sills (No.)	6	Est Unit Rates \$6300/per set)		Est Total Construction Costs	\$31,600
Est Total Rockbolts (LF)	9914	Est Unit Rates \$85/per LF)		Est Total Construction Costs	\$842,690
Est Total Concrete (cy)	0	Est Unit Rates \$110/per CY)		Est Total Construction Costs	\$0
Est Total Shotcrete (cy)	624	Est Unit Rates \$1000/per CY)		Est Total Construction Costs	\$624,000
Est Total Timber Sills (No.)	346	Est Removal Unit Rates \$1600/per set)		Est Total Removal Costs	\$552,800
Est Sub Total for Repairs				\$2,061,290	
				Mobilization (15%)	\$307,684
				Contingency (20%)	\$410,258
				Est Total of Construction Cost	\$2,769,242

ODOT RAIL STUDY  
TUNNEL 16  
Coos Bay Subdivision, Oregon  
MP 721.52 to 721.64

Shannon Wilson Inc

Station		Length, ft	Repair Level	Lining		VIN	Concrete Curb Height above TOR, in	Comments	Repairs	Steel Bars						Concrete		Timber Bars	
From	To			Type	Set Spacing, ft														
0+00	0+00	0		Concrete Portal				North Portal @ MP 721.52 Concrete barrel. Scattered cracks in barrel crown. Scattered drips in crown - ½"-¾" open crack on east sidewall and ½" open crack on west wall at Sta 0+10 - Horizontal crack along west sidewall from Sta 0+11 to 0+55, offset in pieces up to ½". - Horizontal crack ½" open along east sidewall from 0+11 to 0+55 other											
0+00	0+55	55		Concrete Barrel				Shotcrete over steel sets at 2 5'-spacing on concrete curb. - Drips near center line of crown between Sta 1+21 and 1+33 - Damp with discoloration along center line of crown at Sta 3+53 and at Sta 3+63 - Damp spot on west side of crown at Sta 4+00 - Seepage in crown with dripping locally and orange discoloration Dry - Seepage in crown with dripping locally and while to orange discoloration											
0+55	4+10	355		Shotcrete over Steel Sets	2.5	Y	6												
4+10	4+43	33		Shotcrete over Steel Sets	2.5	Y	6												
4+43	4+56	13		Shotcrete over Steel Sets	2.5	Y	6												
4+56	4+88	30		Shotcrete over Steel Sets	2.5	Y	6												
4+88	5+00	14		Shotcrete over Steel Sets	2.5	Y	6												
5+00	5+52	52		Shotcrete over Steel Sets	2.5	Y	6												
5+52	5+78	26		Shotcrete over Steel Sets	2.5	Y	6												
5+78	6+33	55		Concrete Barrel															
6+33	6+33	0		Concrete Portal					South Portal @ MP 721.64										
Total Length (ft)		633									Total		0		0		0		
Repair Level 5 (ft)		633																	

Repair Level	
2	Repairs should be completed immediately to <6 months
3	Repairs should be completed in 0 to 12 months
4	Repairs should be completed in 12 - 30 months
5	Repairs should be completed in 30 - 48 months
6	No immediate repairs required based on the current conditions

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5									
Est. Total Steel Sets (No.)	0	Est. Unit Rates \$5300/per set)	Est. Total Construction Costs	\$0					
Est. Total Rockbolts (LF)	0	Est. Unit Rates \$85/per LF)	Est. Total Construction Costs	\$0					
Est. Total Concrete (CY)	0	Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0					
Est. Total Shotcrete (CY)	0	Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$0					
Est. Total Timber Sets (No.)	0	Est. Removal Unit Rate \$1800/per set)	Est. Total Removal Costs	\$0					
				Est. Sub Total for Repairs	\$0				
				Mobilization (15%)	\$0				
				Contingency (20%)	\$0				
				Est. Total of Construction Cost	\$0				

Table 5



ODOT RAIL STUDY  
 TUNNEL 17  
 Coos Bay Subdivision, Oregon  
 MP 727.70 to 727.83

Shannon Wilson Inc

Station		Length, ft	Repair Level	Lining		VIN	Concrete Curb		Comments	Repairs	Steel Bars		Rockbolts		Shotcrete		Concrete		Timber Sols		Comments
From	To			Type	Set Spacing, ft		Height above TOR, in	R			No	Rows	LF	ft <sup>3</sup>	cy	ft <sup>3</sup>	cy	R	No		
0+00	0+00	0		Concrete Portal					North Portal @ MP 727.70 Concrete barrel - Damp from Sta 0+25 to 1+28 - Crack in E-sidewall at Sta 0+52. - Crack in W-sidewall at Sta 0+57. - Crack at center line of crown from -Sta 0+72 to 1+28. - Ditches along both sides are filled with silt and gravel. depth ditches appear to be at least 18" below rail Shotcrete over steel sets at narrow spacing on concrete curb. Application is relatively thin (~4"-thick) Existing timber lagging was covered with shotcrete possibly. Track is sunk in mud - Flowing water from ¼-inch on E-sidewall (est. 5-10 gal/min) at Sta 1+39												v muddy ballast Sta 0+00 to 2+50
				Concrete Barrel																	
1+28	1+43	15		Shotcrete over Steel Sets	1-1.5	Y	6		Shotcrete over steel sets at 2'- spacing on concrete curb. Thickness of application is between 3" and 6", thinner in crown generally. Abundant dips; shotcrete is discolored at many locations. Tunnel dries up around Sta 2+30 generally. Very muddy track.												
1+43	2+45	102		Shotcrete over Steel Sets	2	Y	6		Shotcrete over steel sets at 4'- spacing on concrete curb. Thickness of application is between 4" and 6", thinner in crown generally. Scattered dips; shotcrete is discolored in many locations. Track dries up and ballast starts to become visible between Sta 2+50 and 2+60												
2+45	3+39	94		Shotcrete over Steel Sets	4	Y	6		Shotcrete over steel sets at 2'- spacing on concrete curb. Thickness of application is between 4" and 6", thinner in crown generally.												
3+39	3+65	26		Shotcrete over Steel Sets	2	Y	6		Shotcrete over steel sets at 4'- spacing on concrete curb. Thickness of application is between 4" and 6", thinner in crown generally. Dry												
3+65	4+29	64		Shotcrete over Steel Sets	4	Y	6		Shotcrete over steel sets at 7'- spacing on concrete curb. Thickness of application is between 4" and 6", thinner in crown generally.												
4+29	4+52	23		Shotcrete over Steel Sets	2	Y	6		Shotcrete over steel sets at 4'-spacing on concrete curb with timber lagging. Dry												
4+52	4+60	8		Timber Sols on Concrete Curb with Timber Lagging	4	Y	6			(Current conditions of timber lining and wood foot blocks are fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and application of 4"-thick steel fiber reinforced shotcrete.)											
4+60	5+80	120		Timber Sols on Wood Foot Blocks with Timber Lagging	4	N			Timber sets at 4'-spacing on wood foot blocks with timber lagging. Dry Timber and foot blocks are in fair to good condition. Foot blocks are covered in ballast and debris along west sidewall. Backrock appears to be less than 2' behind timber lagging (lagging almost complete throughout section) Timber sets at 4'-spacing on wood foot blocks with timber lagging. Dry Timber and foot blocks are in fair to good condition generally. Lagging fell out in west sidewall, rotted at bottom, some rock debris behind lagging. Exposed Backrock appears to be massive and competent												
5+80	5+85	15	Timber Sols on Wood Foot Blocks with Timber Lagging	4	N			Timber sets at 4'- spacing on wood foot blocks with timber lagging. Dry Timber and foot blocks are in fair to good condition generally. Tracks start to become muddy again at around Sta 6+20 with signs of pumping													
5+85	6+44	49	Timber Sols on Wood Foot Blocks with Timber Lagging	4	N			Timber sets at 4'-spacing on concrete curb with timber lagging. Focusing of pools embedded in concrete curb one inch. Dry													
6+44	7+29	85	Timber Sols on Concrete Curb with Timber Lagging	4	Y	6		Timber sets at 4'-spacing on wood foot blocks with timber lagging. Dry. Timber and foot blocks are in fair to good condition generally. Tracks are in very poor condition and totally sunk in wet mud, seeping (flowed?) track at approximately Sta 7+50												v muddy ballast Sta 7+00 to 11+00	
7+29	7+68	39	Timber Sols on Wood Foot Blocks with Timber Lagging	4	N			Timber sets at 4'-spacing on wood foot blocks with timber lagging - East sidewall. Rotted lagging with debris piled up 6'-12" high behind it, one pool is rotted. Backrock is exposed roughly 2' behind lagging													
7+68	7+80	12	Timber Sols on Wood Foot Blocks with Timber Lagging	4	N			Timber sets at 4'-spacing on wood foot blocks with timber lagging. Dry. Timber and foot blocks are in fair to good condition generally. Severe drainage problem throughout this section. Tracks, ditches and footings of timber sets are entirely covered in wet mud													
7+80	8+52	72	Timber Sols on Wood Foot Blocks with Timber Lagging	4	N																

Table 6

ODOT RAIL STUDY  
TUNNEL 17  
Coos Bay Subdivision, Oregon  
MP 727.70 to 727.83

Shannon Wilson, Inc.

Station	To	Length, ft	Repair Level	Lining	Type	Set Spacing, ft	V/N	Concrete Curb	Height above TOR, in	Comments	Repairs	Steel Sels	Rockbolts	Shotcrete	Concrete	Timber Sels	Comments		
From												ft	No	ft	sq	ft	No	ft/348 Yards	
8+52	8+69	17	3	Timber Sels on Wood Foot Blocks with Timber Lagging	Shotcrete over Bedrock	4	N			Timber sels at 4'-spacing on wood foot blocks with timber lagging. Entire section sagged 12 inches wood boards were used to strap sels together foot blocks are crushed Bedrock appears to be close behind lagging	Remove timber sels and timber lagging and clean area from rock debris. Install rockbolts (rockbolt rows at 5'-spacing, five 12'-long rockbolts per row). Apply shotcrete to the desired thickness of 4 inches		3	180	1224	15	17	5	
8+69	9+03	34				N						Shotcrete over bedrock. Dry Shotcrete over bedrock concrete curb is present along both sides of tunnel. Track appears to be lowered in this section							
9+03	10+54	151				Y	12					Shotcrete over steel sels on concrete curb at 2'-spacing							
10+54	11+45	91		Shotcrete over Steel Sels	Concrete Barrel	2	Y	12		Drip at Sta 11+10									
11+45	12+00	55									Concrete barrel Sagging in crown at contact of barrel and steel sels								
12+00	12+00	0									South Portal @ MP 727.83								
Total Length (ft)		1200										0	0	8	480	3168	38	44	
Repair Level 3 (ft)		17																	
Repair Level 4 (ft)		12	59																
Repair Level 4-5 (ft)		19																	
Repair Level 5 (ft)		1156																	
COST ESTIMATE FOR REPAIR LEVELS 1 TO 5																			
Est Total Steel Sels (No)										0	(Est Unit Rates \$8,300/per sel)	Est Total Construction Costs	\$0						

Total	0	0	8	480	3168	38	0	0	44	14	
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Est. Total Steel Sels (No.)	0	Est. Unit Rates \$5,300/per sel)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	480	Est. Unit Rates \$85/per LF)	Est. Total Construction Costs	\$40,800
Est. Total Concrete (cy)	0	Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (cy)	38	Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$38,000
Est. Total Timber Sels (No.)	14	Est. Removal Unit Rate \$1600/per sel)	Est. Total Removal Costs	\$22,400

Est. Sub Total for Repairs \$101,200

Mobilization (15%) \$15,180

Contingency (20%) \$20,240

Est. Total of Construction Cost \$136,620

- Repair Level
- Repairs should be completed immediately to <3 months
- 2
- Repairs should be completed in 0 to 12 months
- 3
- Repairs should be completed in 12 - 30 months
- 4
- Repairs should be completed in 30 - 48 months
- 5
- No immediate repairs required based on the current conditions

ODOT RAIL STUDY  
TUNNEL 18  
Coos Bay Subdivision, Oregon  
MP 734.48 to 734.77

Shannon Wilson, Inc.


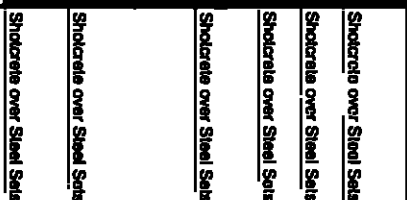

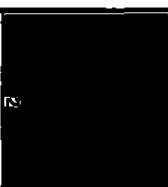


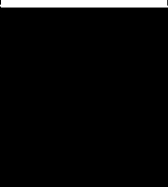
Station		Length, ft	Repair Level	Lining		Concrete Curb		Comments	Repairs	Steel Sails		Rockbolts		Shotcrete		Concrete		Timber Sails		Comments
From	To			Type	Set Spacing, ft	Y/N	Height above TOR, in			R	No	Rows	LF	ft <sup>3</sup>	cy	ft <sup>3</sup>	cy	R	No	
0+00	0+00	0		Shotcrete over Steel Sails	2	Y	12	Shotcrete over Steel Sails at 2'-spacing on concrete curb. Shotcrete -6" thick over steel sails. Dry												Railbed v dirty Sta 0+00 to 4+00
0+00	1+02	102		Shotcrete over Steel Sails	2	Y	12	Shotcrete over Steel Sails at 4'-spacing on concrete curb. Shotcrete -6" thick over steel sails. Dry												
1+02	4+00	298		Shotcrete over Steel Sails	4	Y	12	Shotcrete over Steel Sails at 4'-spacing on concrete curb. Shotcrete -6" thick over steel sails. Dry. Gradual change in curb height beginning at the North Portal (12" above TOR to 0" above TOR)												
4+00	4+12	12		Shotcrete over Steel Sails	4	Y	0	Shotcrete over Steel Sails at 2'-spacing on concrete curb. Shotcrete -6"-6"-thick over steel sails. Dry generally												
4+12	6+04	282		Shotcrete over Steel Sails	2	Y	0 and 6	- Crack along center line of crown between Sta 5+70 and 6+30 dip from existing drain in crown adjacent to crack at Sta 5+82												
6+04	6+05	211		Shotcrete over Steel Sails	4	Y	6 and 0	- Abrupt change in curb height, raised 6" at Sta 5+84												
6+05	6+22	17		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Bottom of timber posts and/or wood footing blocks are severely deteriorated, show signs of crushing, squeezing, and/or shifing, and/or are cracked. Sails sagging by 6" to 12" along east wall	Replacement of wood footing blocks (and bottom of timber posts when deteriorated). Establish new footing for timber sails with shotcrete on bedrock or a minimum of 2 ft below top of rail, whichever is shallower			3	180	1224	15			17	5	
6+22	6+36	14		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Bottom of timber posts and/or wood footing blocks show initial stages of deterioration, but do not show signs of advanced distress or movement	Remove existing timber lining, install rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and apply 4"-thick steel fiber reinforced shotcrete)			3	180	1008	12			14	5	
6+36	6+60	24		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Bottom of timber posts and/or wood footing blocks are severely deteriorated, show signs of crushing, squeezing, and/or shifing, and/or are cracked. Posts settled 12" in many areas on both sides of tunnel	Replacement of wood footing blocks (and bottom of timber posts when deteriorated). Establish new footing for timber sails with shotcrete on bedrock or a minimum of 2 ft below top of rail, whichever is shallower			5	300	1728	21			24	7	
6+60	10+76	118		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Six posts on the east side buckled out at the bottom and shifted into tunnel by up to 2 ft (smallest distance to nearest rail is 3.5 ft, measured at top of rail). Rock fragments and crushed rotted timber lagging is caught behind displaced timber posts. Bedrock is exposed locally	Remove existing timber lining, install rockbolts (row-spacing 5 feet, five 15'-long rockbolts per row) and apply 6"-thick steel fiber reinforced shotcrete)			24	1440	8496	106			118	31	
10+76	11+10	32		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Bottom of timber posts and/or wood footing blocks are severely deteriorated, show signs of crushing, squeezing, and/or shifing, and/or are cracked. Sagged posts on east side between Sta 11+10 and 12+00	Installation of six replacement steel sails, installation of rockbolts (rockbolt rows at 5'-spacing, six 15'-long rockbolts per row). Application of shotcrete to the desired thickness of 6". Footing blocks and bottom of posts of the two sails to the south of the failed area need to be cut and removed, and then replaced with shotcrete along the east side			6	640	2304	42			32	9	No overall change in tunnel conditions
11+10	12+60	170		Timber Sails on Wood Foot Blocks with Timber Lagging	4	N		Bottom of timber posts and/or wood footing blocks are severely deteriorated, show signs of crushing, squeezing, and/or shifing, and/or are cracked. Sagged posts on west side between Sta 11+00 and 11+20, Sta 11+72 and 11+76, and Sta 12+72 and 12+80	Cutting and removal of deteriorated wood footing blocks and rotted bottom sections of timber posts. Establish new footing for timber sails with shotcrete on bedrock or a minimum of 2 ft below top of rail, whichever is shallower			34	2040	12240	154			170	44	

Table 7

ODOT RAIL STUDY  
TUNNEL 18  
Coos Bay Subdivision, Oregon  
MP 734.48 to 734.77

Shannon Wilson Inc

Station		Length, ft	Repair Level	Lining		Concrete Curb	Comments	Repairs	Steel Sails		Rockbolts		Shotcrete		Concrete		Timber Sails		Comments
From	To			Type	Set Spacing, ft				VIN	Height above TOR, in	ft	No	Rows	LF	ft <sup>2</sup>	cy	ft <sup>2</sup>	cy	
12+80	13+22	42	3 - 4	Timber Sails on Wood Foot Blocks with Timber Lagging	4	N	Bottom of timber posts and/or wood footing blocks show initial stages of deterioration, but do not show signs of advanced distress or movement	Replacement of wood footing blocks (and bottom of timber posts when deteriorated). Establish new footing for timber sets with shotcrete on bedrock or a minimum of 2 ft below top or rail, whichever is shallower OR Remove existing timber lining, install rockbolts (row-spacing 5 foot, five 12'-long rockbolts per row) and apply 4'-thick steel fiber reinforced shotcrete.) Replacement of wood footing blocks (and bottom of timber posts when deteriorated). Establish new footing for timber sets with shotcrete on bedrock or a minimum of 2 ft below top or rail, whichever is shallower OR Remove existing timber lining, install rockbolts (row-spacing 5 feet, five 12'-long rockbolts per row) and apply 4'-thick steel fiber reinforced shotcrete.) (Current conditions of timber lining and wood foot blocks are fair to good generally. However, timber will deteriorate over time and may cause problems in these sections in the future. Replacing the timber lining with rockbolts and steel fiber reinforced shotcrete is recommended in the future in order to maintain the long-term stability of the tunnel. Future repairs should include installation of rockbolts (row-spacing 5 feet five 12'-long rockbolts per row) and application of 4'-thick steel fiber reinforced shotcrete. OR Replacement of wood footing blocks and bottom of timber posts when deteriorated. Establish new footing for timber sets with shotcrete on bedrock or a minimum of 2 ft below top or rail, whichever is shallower.)			8	480	3024	38		42	12		
13+22	14+20	88	2	Timber Sails on Wood Foot Blocks with Timber Lagging	4	N	Timber posts and lagging in fair to good condition				20	1200	7056	88		98	26		
14+20	14+82	62		Timber Sails on Concrete Curb with Timber Lagging	4	Y													
14+82	15+27	45		Timber Sails with Timber Lagging	4	Y	Posts on east side shifted into tunnel at the bottom. Rubble and detached rolled timber lagging and cribbing caught behind posts between Sta 15+20 and 15+27. Timber set is missing at Sta 15+18. Timber lagging rotted and missing locally. Some timber sets are separated at butt joints and from timber lagging. Posts are rotted at the bottom along the east side between Sta 15+00 and 15+10	Removal of ten existing timber sets, installation of rock bolts (rockbolt rows at 5'-spacing six 15'-long rockbolts per row) Application of shotcrete to desired thickness of 6"				9	675	3240	60		45	12	
15+27	15+80	53		Concrete Barrel															
15+80	15+80	0		Concrete Portal			South Portal @ Sta 734.77												
Total Length (ft.)		1580																	

Repair Level 1 (ft.) 77  
Repair Level 2 (ft.) 400  
Repair Level 3-4 (ft.) 83  
Repair Level 5 (ft.) 1020

Repair Level  
2 Repairs should be completed immediately to <6 months  
3 Repairs should be completed in 0 to 12 months  
3 Repairs should be completed in 12 - 30 months  
3 Repairs should be completed in 30 - 48 months  
No immediate repairs required based on the current conditions

COST ESTIMATE FOR REPAIR LEVELS 1 AND 2

Est. Total Steel Sails (No.)	0 (Est. Unit Rates \$6300/per set)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	6075 (Est. Unit Rates \$88/per LF)	Est. Total Construction Costs	\$516,375
Est. Total Concrete (cy)	0 (Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (cy)	482 (Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$482,000
Est. Total Timber Sails (No.)	125 (Est. Removal Unit Rate \$1600/per set)	Est. Total Removal Costs	\$200,000
Est. Sub Total for Level 1 and 2 Repairs			\$1,178,375
		Mobilization (15%)	\$176,756
		Contingency (20%)	\$235,675
Est. Total of Level 1 and 2 Construction Cost			\$1,590,806

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5

Est. Total Steel Sails (No.)	0 (Est. Unit Rates \$6300/per set)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	7035 (Est. Unit Rates \$88/per LF)	Est. Total Construction Costs	\$597,975
Est. Total Concrete (cy)	0 (Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (cy)	538 (Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$538,000
Est. Total Timber Sails (No.)	149 (Est. Removal Unit Rate \$1600/per set)	Est. Total Removal Costs	\$238,400
Est. Sub Total for Repairs			\$1,372,375
		Mobilization (15%)	\$205,856
		Contingency (20%)	\$274,475
Est. Total of Construction Cost			\$1,852,706

Table 7

Station		Length, ft	Repair Level	Lining	Set Spacing, ft	Concrete Curb		Comments	Repairs	Shotcrete						Timber Sids	
From	To					V/N	Height above TOR, in			ft	No	Recess	LF	ft <sup>3</sup>	cu yd	ft <sup>3</sup>	cu yd
0+00	0+00	0	Shotcrete over Steel Sids			Y	1	North Portal @ MP 745+52, Slum slides at the west side of the portal area occurred in the past and resulted in blocked drainage (muddy track and ditches inside the tunnel)  Shotcrete - 6" thick over steel sids, dry, failed tracks Shotcrete - 6" thick over steel sids, dry, failed tracks Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)  to 2+10 (approx.) 6" curved track  - Drainage ditches along both sides of tunnel are in very bad shape and essentially non-functional. Color changes of the shotcrete indicate multiple applications in the past, suggesting repair of previously spalled shotcrete. In general, the present shotcrete cover in the tunnel is in good condition and does not show signs of cracking and advancing deterioration. However, spalling shotcrete was observed at several locations in the tunnel, some associated with small rockfalls. It is recommended to cover exposed bedrock in areas where shotcrete is spalling (sidewalls 2"-thick, spalled areas 4"-thick) within the next 48 to 60 months in order to prevent further weathering and deterioration of bedrock, and potential rockfalls, this will also help to prevent the spalling from progressing into the existing shotcrete application surrounding the spalled areas. Due to the generally thin shotcrete cover now spalling of shotcrete has to be expected in the future at any location throughout the tunnel because of the good condition of the existing shotcrete cover and the dry state of the tunnel bottom.									
0+00	0+35	35	Shotcrete over Steel Sids	2	Y	1											
0+35	0+50	15	Shotcrete over Steel Sids	4	Y	1											
			Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)														
0+50	10+86	1036	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (~ 5'-5.5'), shotcrete fragments on track									
10+86	10+91	5	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
10+91	11+14	23	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (~ 5'x3') associated with rock fall, shotcrete fragments and rock fragments (largest rock ~ 4'x2'x1') on track and in ditches									
11+14	11+20	6	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
11+20	11+31	11	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (~ 5'x2') associated with rock fall, shotcrete fragments and small rock fragments on track									
11+31	11+36	5	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick), color changes in shotcrete (light-gray and dark-gray) suggest multiple applications or previous shotcrete repair work  Signifi dry from E-gonophore at Sta 13+160.  Both ditches are silted up and muddy again starting at around Sta 26+00									
11+36	26+32	1496	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (~ 6'-15'-wide area) associated with rock fall, shotcrete fragments and small rock fragments on ground									
26+32	26+76	44	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
26+76	32+20	544	Shotcrete over Bedrock		N	-	-	Spalling shotcrete across tunnel crown (~ 20'-wide) associated with rock fall, shotcrete fragments and small rock fragments on ground									
32+20	32+40	20	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
32+40	32+70	30	Shotcrete over Bedrock		N	-	-	Areas with spalling shotcrete (8'-5' and 10'-6') in tunnel crown (< 1 0'-thick application), shotcrete fragments on ground, scattered small rock fragments on track at Sta 32+70									
32+70	32+90	20	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
32+90	33+70	80	Shotcrete over Bedrock		N	-	-	Areas with patches of spalling shotcrete in tunnel crown (0.5'-1 0'-thick application), shotcrete fragments on ground (no rock fragments) - track and ditches are very muddy and carry wood debris (condition continues to S-Portal)									
33+70	33+82	22	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
33+82	35+82	170	Shotcrete over Bedrock		N	-	-	Patches of spalling shotcrete in tunnel crown (0.5'-1 0'-thick application) shotcrete fragments and scattered small rock fragments on ground									
35+82	35+90	28	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
35+90	36+82	102	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (< 0.5'-thick application)									
36+82	36+89	7	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
36+89	37+83	96	Shotcrete over Bedrock		N	-	-	Patches of thin spalling shotcrete in tunnel crown									
37+83	37+83	33	Shotcrete over Bedrock		N	-	-	Spalling shotcrete across tunnel crown (~ 20'-wide)									
37+83	38+26	24	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in western half of tunnel crown (~ 10'-wide approx.)									
38+26	38+50	18	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
38+50	38+68	16	Shotcrete over Bedrock		N	-	-	Two 5' to 10'-wide patches of thin spalling shotcrete in tunnel crown									
38+68	38+84	16	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
38+84	38+94	37	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (5'x4')									
38+94	39+21	7	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
39+21	39+28	5	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (5'x4')									
39+28	39+33	23	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
39+33	39+56	23	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (3'x2')									
39+56	39+59	3	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
39+59	39+87	38	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (5'x5')									
39+87	40+02	5	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (5'x5')									
40+02	41+00	98	Shotcrete over Bedrock		N	-	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5" to 2.0" thick)									
41+00	41+07	7	Shotcrete over Bedrock		N	-	-	Spalling shotcrete in tunnel crown (7'x5')									

ODOT RAIL STUDY  
TUNNEL 19  
Coos Bay Subdivision, Oregon  
MP 745.62 to 746.41

Shannon Wilson Inc.

Station		Length, ft	Repair Level	Lining		Concrete Curb	Comments	Repairs	Steel Sides		Rockwalls		Shotcrete		Concrete		Timber Sides		
From	To			Type	Set Spacing, ft				VIN	Height above TOR, in	ft	no	ft	no	sq	cy	sq	cy	ft
41+07	41+38	31		Shotcrete over Bedrock		N	-	Shotcrete applied over bedrock, shotcrete cover is thin in general (0.5' to 2.0' thick) to 41+38. Seepage at springline along W-side wall. Spalling shotcrete in tunnel crown (8'x4'). Concrete Slant South Portal @ MP 746.41 Slant slides at the east side of the portal area occurred in the past and resulted in blocked drainage (muddy track and ditches inside the tunnel)											
41+38	41+47	9		Shotcrete over Bedrock		N	-							648	6				
41+47	42+02	55		Concrete		N	-												
42+02	42+02	0		Concrete		N	-												
Total Length (ft)		4202																	
Repair Level 4.5 (ft)		281																	
Repair Level 5 (ft)		3811																	

Repair Level  
2 Repairs should be completed immediately to <6 months  
3 Repairs should be completed in 0 to 12 months  
3 Repairs should be completed in 12 - 30 months  
3 Repairs should be completed in 30 - 48 months  
3 No immediate repairs required based on the current conditions

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5									
Est. Total Steel Sides (No.)	0	Est. Unit Rates \$3300/per set)	Est. Total Construction Costs	\$0					
Est. Total Rockholes (LF)	0	Est. Unit Rates \$85/per LF)	Est. Total Construction Costs	\$0					
Est. Total Concrete (CY)	0	Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0					
Est. Total Shotcrete (cy)	191	Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$191,000					
Est. Total Timber Sides (No.)	0	Est. Removal Unit Rate \$1800/per set)	Est. Total Removal Costs	\$0					
				Est. Sub. Total for Repairs	\$191,000				
				Mobilization (15%)	\$28,650				
				Contingency (20%)	\$38,200				
				Est. Total of Construction Cost	\$257,850				

ODOT RAIL STUDY  
 TUNNEL 20  
 Coos Bay Subdivision, Oregon  
 MP 750.12 to 750.28

Station From	To	Length, ft	Repair Level	Lining		Concrete Curb	Comments	Repairs	Steel Sails		Rockbolts	Shotcrete		Concrete		Timber Sails					
				Type	Set Spacing, ft				Y/N	Height above TOR, in		ft	No	ft <sup>3</sup>	cy	ft <sup>3</sup>	cy	ft	No		
0+00	0+00	0		Concrete Portal		N	North Portal @ MP 750.12 Generally dry to damp, - Thin crack with seepage on west sidewalk at ~Sta 0+09 - Open crack (1/4"-wide) in east sidewalk at ~Sta 0+25 - Scattered dips around Sta 0+42														
0+00	0+54	54		Concrete Barrel		N	Generally thin shotcrete cover, especially in crown. Shotcrete is not reinforced. No cracks observed														
0+54	1+53	99		Shotcrete over Bedrock		N	- Bedrock exposed in crown between Sta 1+43 and 1+53 Shotcreted steel sets to bridge overbreak/cave-in area (steel sets do not touch and support ground in crown). Carved-in section is 10'-15' high in crown. Loose bedrock material, originating from open ground behind shotcreted steel sets, piled up at the bottom of the east sidewalk at each end of the section. Dry														
1+53	1+73	20	3	Shotcrete over Steel Sails	3.5	Y	Generally thin shotcrete cover, especially in crown. Shotcrete is not reinforced. Dry in general. - 9 rockbolts scattered in crown between Sta 1+75 - 1+85 - Spalled shotcrete in crown (2"x2") at Sta 2+19 - 5 rockbolts scattered in crown between Sta 2+73 and 2+28 drip above western springline - Spalled shotcrete (2"x2") along west wall at Sta 2+48 2+51 and 2+58. - Seepage in west wall at roughly ~Sta 2+70 (leak?). - Spalled shotcrete in crown (3" x 1") at Sta 4+11		8	2		576	8	672	124						
1+73	5+08	335		Shotcrete over Bedrock		N	Generally thin shotcrete cover, especially in crown. Shotcrete is not reinforced. Dry in general. - 5 to 7'-high overbreak area between Sta 5+08 and 5+38 across crown, shotcreted from Sta 5+08 to 5+24 with some spalling at Sta 5+10 (4"x4"), Sta 5+14 (3"x2"), Sta 5+21 (5"x2"), and Sta 5+22 (1"x3") large spalling area with bedrock exposed between Sta 5+24 and 5+38, associated with scattered rock fall as indicated by debris on tunnel floor. - Spalling on east wall at Sta 5+37 (2"x2") and Sta 5+49 (4"x8") - Spalled shotcrete in crown at Sta 5+48 (3"x3")	Due to relatively large size of areas with spalled shotcrete (in addition to occasional rockfalls evidently) it is recommended to repair and secure the area with a 2" (over existing shotcrete in sidewalks) to 4" (over exposed bedrock)-thick steel fiber reinforced shotcrete cover													
5+08	5+53	45		Shotcrete over Bedrock		N	Shotcreted steel sets to bridge overbreak/cave-in area (steel sets do not touch and support ground in crown, locally). Overbreak is 3'-5" high in crown. No signs of instabilities or rock falls were observed. Section consists of 12 steel sets at 4'-spacing generally last 3 sets at the south end of section at 3'-spacing	Establish a butthead on both ends of the steel set section and backfill the void space with concrete material. This may require the application of shotcrete at each end of the section					3240	30							
5+53	5+87	44		Shotcrete over Steel Sails	4' and 3'	Y	Generally thin shotcrete over bedrock. Scalloped small drip locations. Muddy track							?	7	1056	117				
5+87	6+32	35		Shotcrete over Bedrock		N	Spalled shotcrete on east wall (8"x22")	Cover exposed bedrock in areas where shotcrete is spalling (including sidewalks, 2"-thick spalled areas, 4"-thick)													
6+32	6+54	22		Shotcrete over Bedrock		N	Generally thin shotcrete over bedrock. Dry, Muddy Track						1584	14							
6+54	7+14	60		Shotcrete over Bedrock		N	Spalled shotcrete in crown (12"x8")	Cover exposed bedrock in areas where shotcrete is spalling (including sidewalks, 2"-thick spalled areas, 4"-thick)													
7+14	7+28	12		Shotcrete over Bedrock		N	Generally thin shotcrete over bedrock						864	8							
7+28	8+20	94		Shotcrete over Bedrock		N	Spalled shotcrete in crown at Sta 7+82 (3"x4")														
8+20	8+74	54		Concrete Barrel		N	- Seams remaining behind concrete barrel at contact at Sta 8+20. Drip zone in crown at Sta 8+41.														
8+74	8+74	0		Concrete Portal		N	Seepage from crack in west sidewalk at Sta 8+45														
Total Length (ft)									874	Total		8	2	0	0	6264	60	1728	241	0	0

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5

Est Total Steel Sails (No)	2	Est Total Construction Costs	\$10,800
Est Total Rockbolts (LF)	0	Est Total Construction Costs	\$0
Est Total Concrete (cy)	241	Est Total Construction Costs	\$28,510
Est Total Shotcrete (cy)	80	Est Total Construction Costs	\$90,000
Est Total Shotcrete (No)	0	Est Total Construction Costs	\$0
Est Total Timber Sails (No)		Est Sub Total for Repairs	\$87,110
		Materialization (15%)	\$14,567
		Contingency (20%)	\$19,422
		Est Total of Construction Cost	\$131,099

Repair Level 1	Repairs should be completed immediately to <8 months
2	Repairs should be completed in 0 to 12 months
3	Repairs should be completed in 12 - 30 months
4	Repairs should be completed in 30 - 48 months
5	No immediate repairs required based on the current conditions

ODOT Rail Study  
 TUNNEL 21  
 Coos Bay Subdivision, Oregon  
 MP 751.21 to 751.30

Shannon Wilson, Inc

Station From To	Length, ft	Repair Level	Lining		Concrete Curb Height above TOR, in	Comments	Repairs	Steel Bars		Reinforcing Bars		Shotcrete		Concrete		Timber Sills		
			Type	Set Spacing, ft				Y/N					ft	no	Rein	LF	ft <sup>3</sup>	cy
0+00	0+00	0	Concrete Portal		N	North Portal @ MP 751.21 In general dry. Thin crack across concrete barrel with some seepage at Sta 0+10.												
0+00	0+55	55	Concrete Barrel		N	Shotcrete cover generally in good condition. Shotcrete was applied after barrel fire. Shotcrete is steel fiber reinforced. Exposed bedrock at bottom 3' of sidewalls (weathered sandstone with scattered siltstone layers), bedrock debris and shotcrete rebound piled along both sidewalls. - 3 rockbolts in crown at Sta 0+70 - 4 rockbolts in crown east of center line at ~Sta 1+04 - Thin crack in crown. 4 rockbolts in crown at ~Sta 2+40	Extend 3"-thick shotcrete cover to the base of the sidewalls in order to prevent excessive weathering and deterioration of exposed sandstone which could result in undermining and spalling of the existing shotcrete application.											
0+55	3+63	308	Shotcrete over Bedrock		N								3686	34				
3+63	4+24	61	Shotcrete over Steel Sills	2.5	Y	6												
4+24	4+78	54	Concrete Barrel		N													
4+78	4+78	0	Concrete Portal		N													
Total Length (ft)		478																
Repair Level 4.5 (ft)		308																
Repair Level 5 (ft)		170																
Total								0	0	0	0	3696	34	0	0	0	0	0

COST ESTIMATE FOR REPAIR LEVELS 1 TO 5

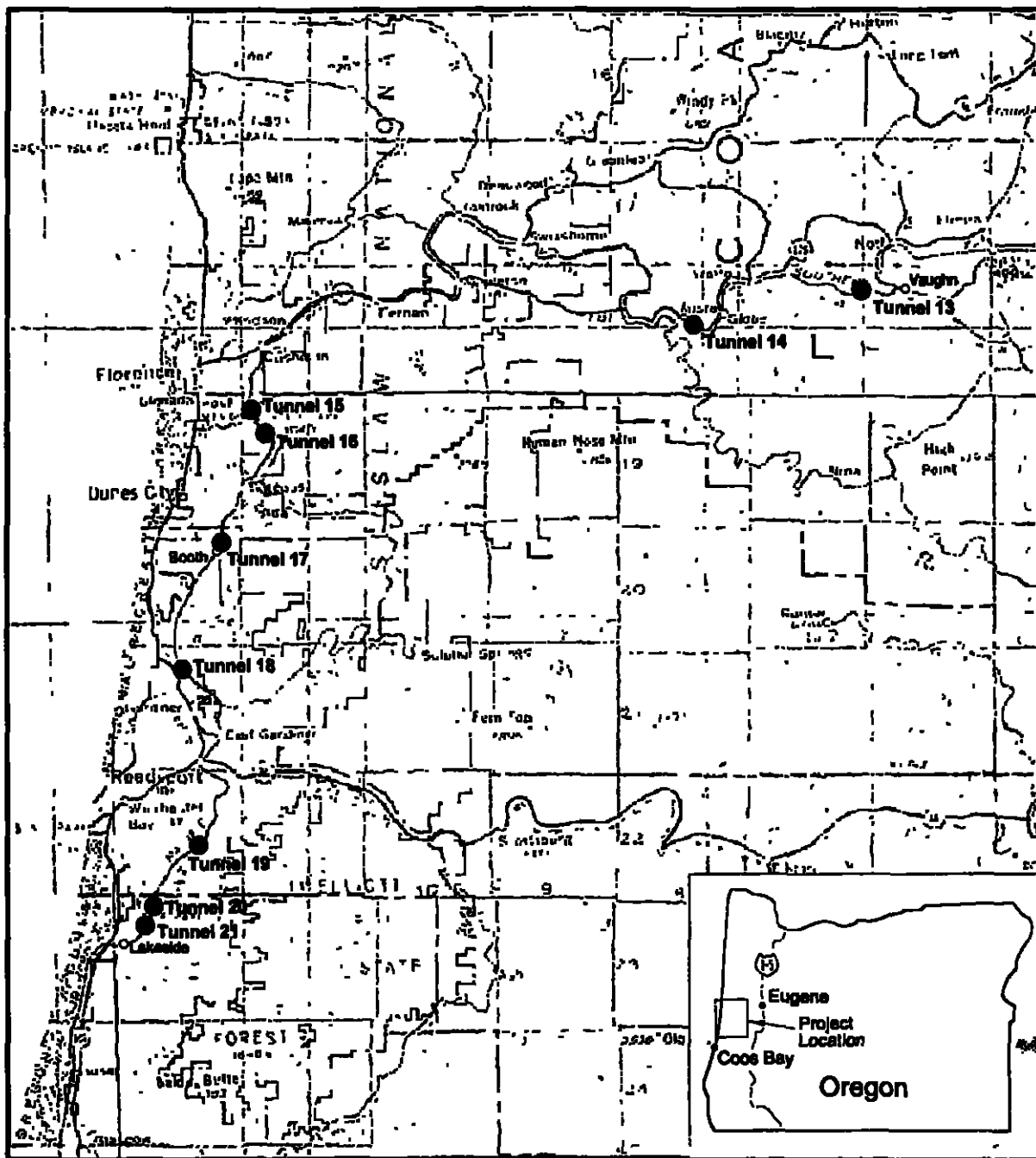
Est. Total Steel Sills (No.)	0	Est. Unit Rates \$8300/per set)	Est. Total Construction Costs	\$0
Est. Total Rockbolts (LF)	0	Est. Unit Rates \$85/pair (LF)	Est. Total Construction Costs	\$0
Est. Total Concrete (CY)	0	Est. Unit Rates \$110/per CY)	Est. Total Construction Costs	\$0
Est. Total Shotcrete (CY)	34	Est. Unit Rates \$1000/per CY)	Est. Total Construction Costs	\$34,000
Est. Total Timber Sills (No.)	0	Est. Removal Unit Rate \$1600/per set)	Est. Total Removal Costs	\$0
			Est. Sub Total for Repairs	\$34,000
			Mobilization (15%)	\$5,100
			Contingency (20%)	\$6,800
			Est. Total of Construction Cost	\$45,900

2	Repairs should be completed immediately to <6 months
2	Repairs should be completed in 0 to 12 months
3	Repairs should be completed in 12 - 30 months
3	Repairs should be completed in 30 - 48 months
3	No immediate repairs required based on the current conditions



# ODOT RAIL STUDY ESTIMATED CONSTRUCTION COST SUMMARY Coos Bay Subdivision, Oregon

Tunnel #	Milepost @ N-Portal	Length (ft.)	Repair Level 1&2 (incl.2-3)					Repair Level 1 to 5					Est. Total Construction Cost	
			Steel Sets (No.)	Rockbolts (LF)	Shotcrete (cy)	Timber Sets (No.)	Concrete (cy)	Steel Sets (No.)	Rockbolts (LF)	Shotcrete (cy)	Timber Sets (No.)	Concrete (cy)		
Tunnel 13	689.47	2498	-	-	-	38	-	\$988,763	-	-	1243	-	-	\$3,048,178
Tunnel 14	681.09	471	-	-	-	-	-	-	-	66	-	-	-	\$89,100
Tunnel 15	720.73	2143	6	2600	84	72	-	\$608,480	6	9814	624	346	-	\$2,769,242
Tunnel 16	721.52	833	-	-	-	-	-	-	-	-	-	-	-	-
Tunnel 17	727.70	1200	-	-	-	-	-	-	-	-	-	-	-	-
Tunnel 18	734.48	1580	-	-	-	-	-	-	-	480	-	14	-	\$136,820
Tunnel 19	745.82	4202	-	8075	482	125	-	\$1,580,808	-	7035	536	149	-	\$1,852,706
Tunnel 20	750.12	874	-	-	-	-	-	-	-	-	191	-	-	\$257,850
Tunnel 21	751.21	478	-	-	-	-	-	-	2	-	80	-	241	\$131,088
Totals		14077	6	10745	975	235	-	\$3,069,049	8	26555	36758	658	241	\$8,330,695



0 7.5 15  
Approximate Scale in Miles

**NOTE**

Map adapted from electronic CD ROM USGS topographic map by TOPO! ©2000 National Geographic Holdings.

ODOT Rail Division  
Rail Tunnels Assessments  
CORP Coos Bay Division Review

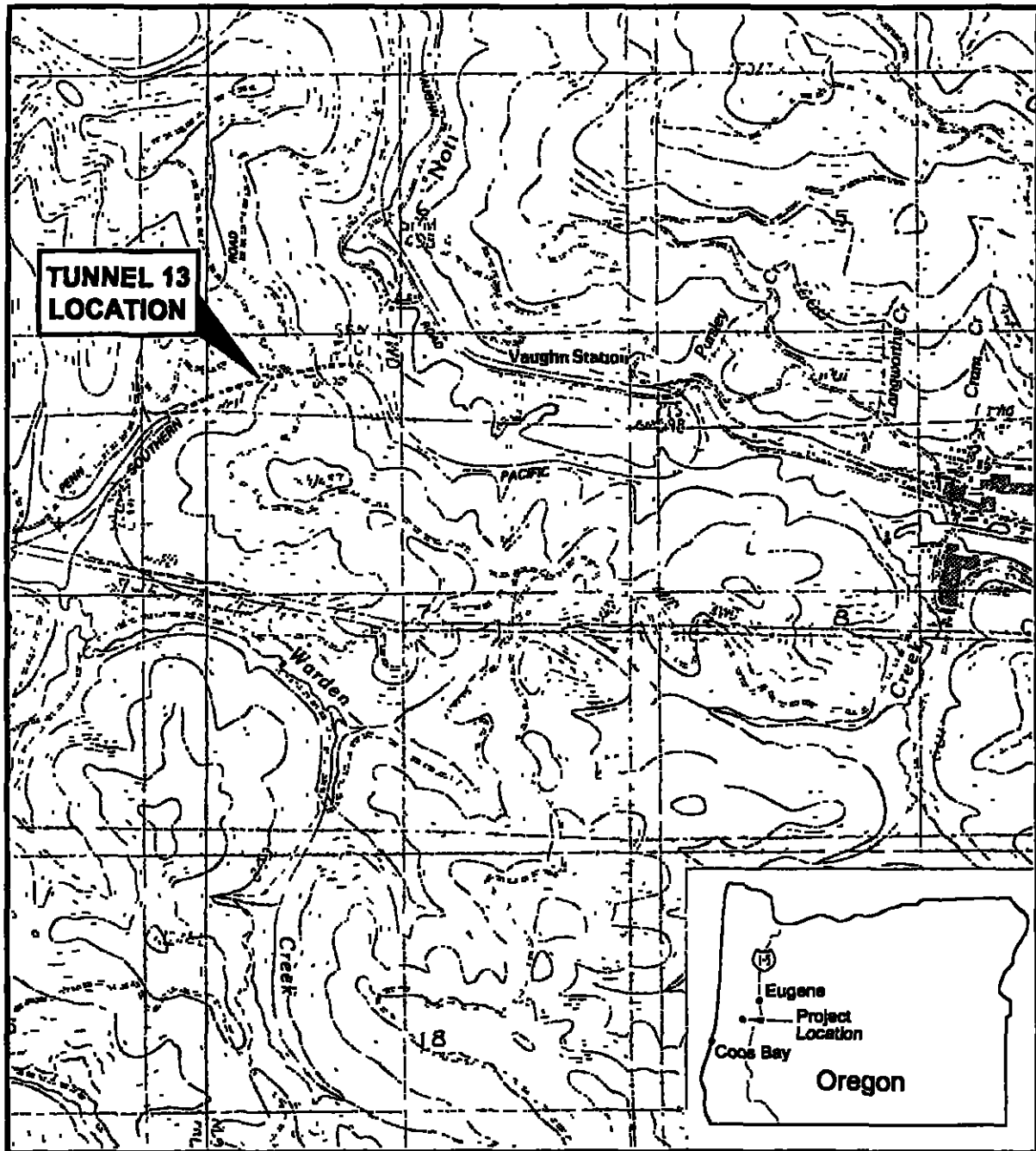
**VICINITY MAP**

September 2008

24-1-03505-002

**SHANNON & WILSON, INC.**  
Geotechnical and Environmental Consultants

**FIG. 1**



**NOTE**

Map adapted from 1:24,000 USGS topographic map of Noti, OR quadrangle, Provisional Edition, dated 1984.

ODOT Rail Division  
Rail Tunnels Assessments  
CORP Coos Bay Division Review

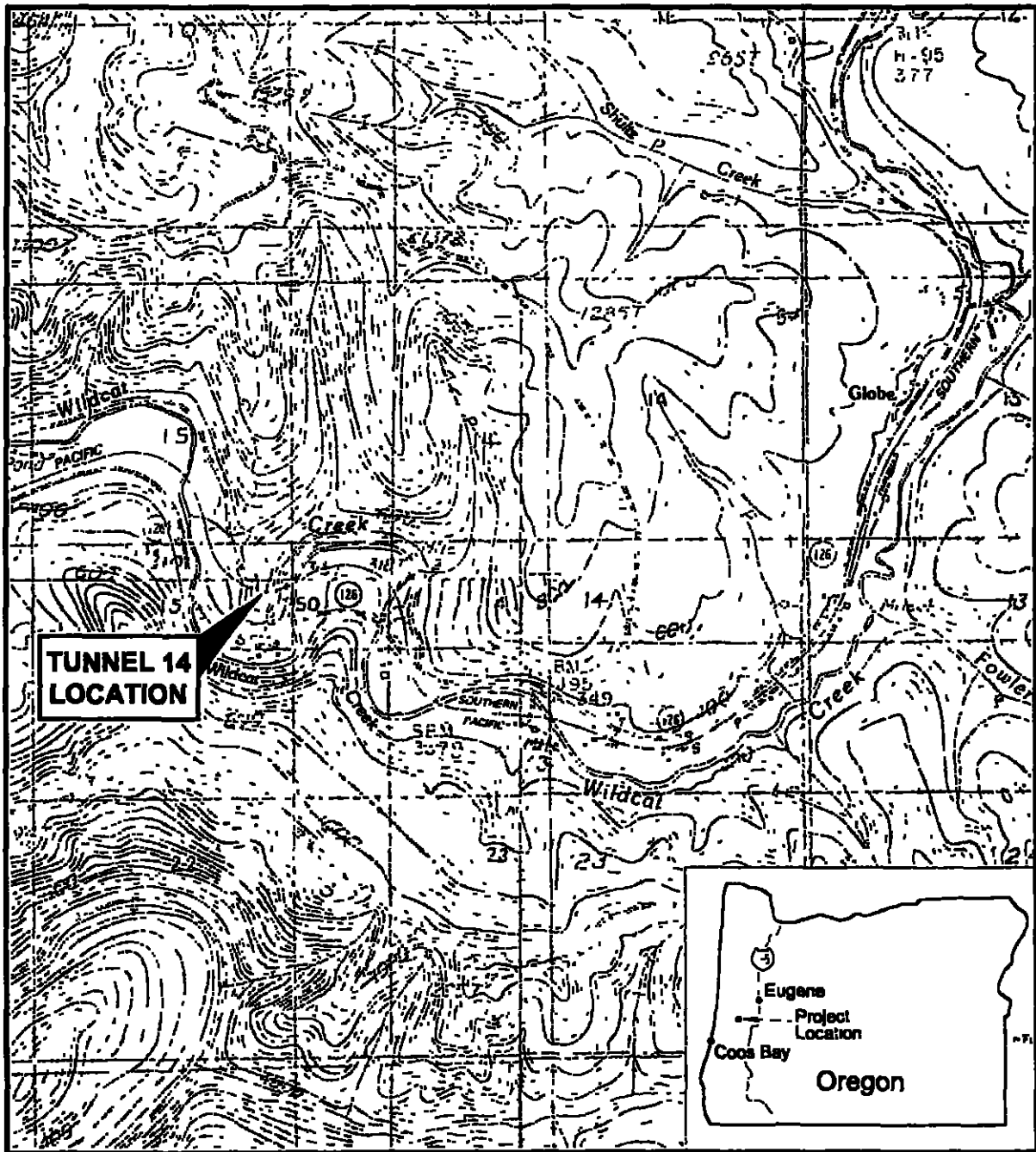
**VICINITY MAP TUNNEL 13**

September 2008

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**FIG. 2**



**NOTE**

Map adapted from 1:24,000 USGS topographic maps of Clay Creek; Walton; Greenleaf, and Roman Nose Mtn, OR quadrangles, all Provisional Editions, all dated 1984.

ODOT Rail Division  
Rail Tunnels Assessments  
CORP Coos Bay Division Review

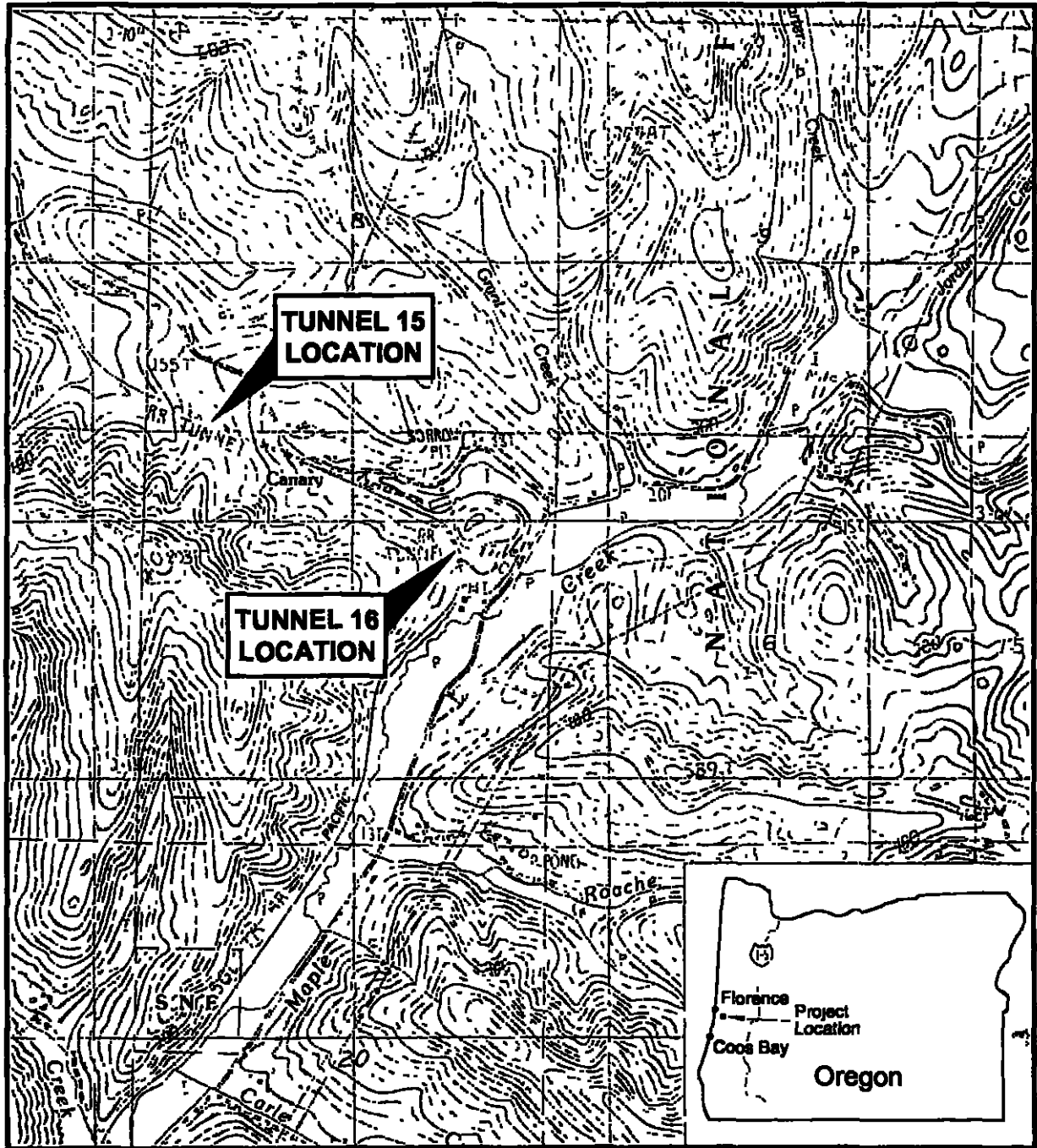
**VICINITY MAP TUNNEL 14**

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**FIG. 3**



**NOTE**

Map adapted from 1:24,000 USGS topographic map of Florence, OR quadrangle, Provisional Edition, dated 1984.

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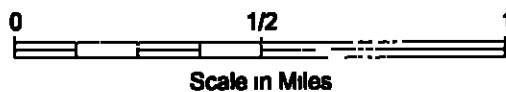
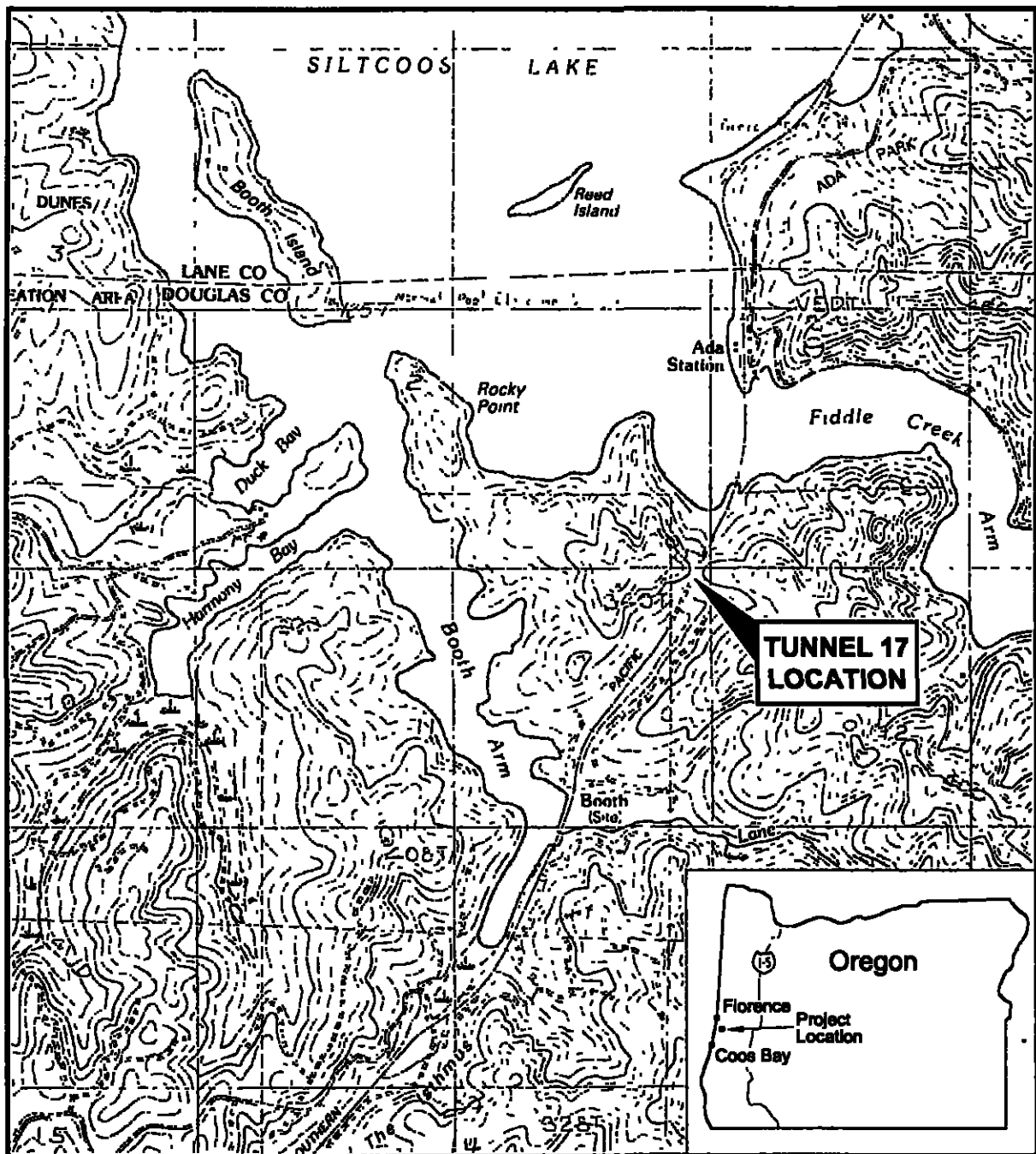
**VICINITY MAP TUNNELS 15 AND 16**

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**FIG. 4**



**NOTE**

Map adapted from 1:24,000 USGS topographic map of Fivemile Creek, OR quadrangle, Provisional Edition, dated 1984.

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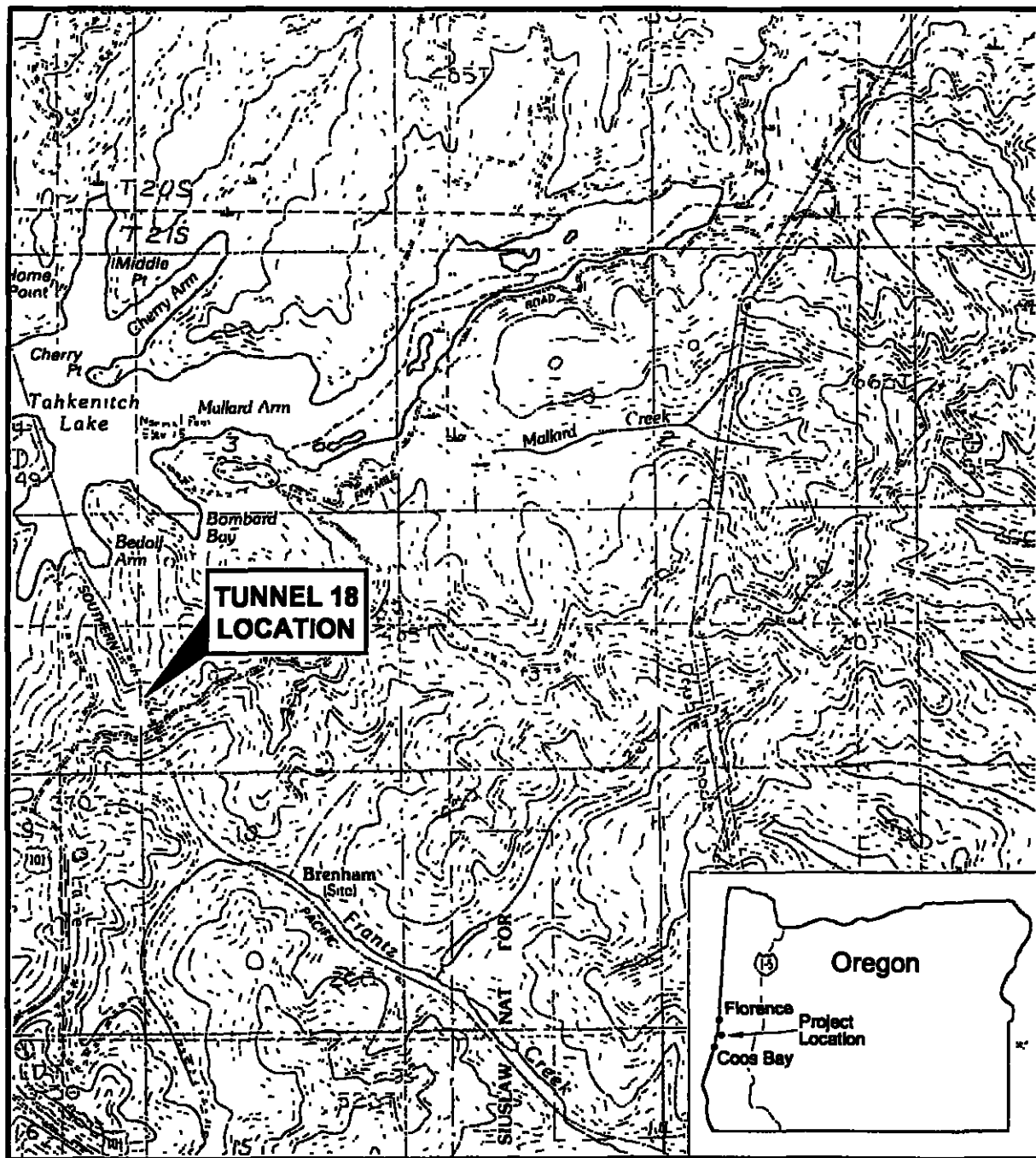
**VICINITY MAP TUNNEL 17**

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**FIG. 5**



**NOTE**

Map adapted from 1:24,000 USGS topographic map of Fivemile Creek, OR quadrangle, Provisional Edition, dated 1984.

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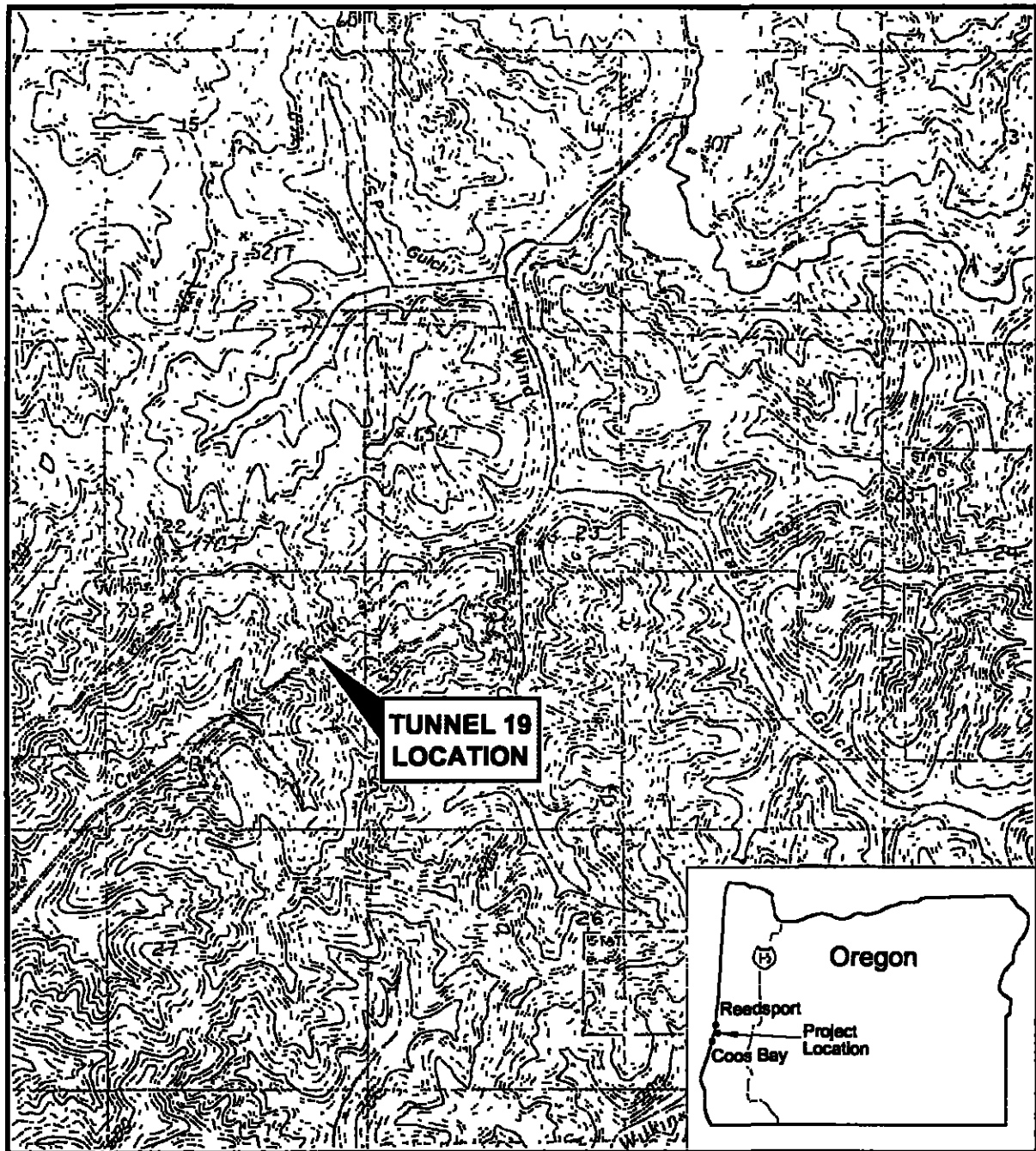
**VICINITY MAP TUNNEL 18**

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**FIG. 6**



**NOTE**

Map adapted from 1:24,000 USGS topographic map of Reedsport, OR quadrangle, Provisional Edition, dated 1985.

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**VICINITY MAP TUNNEL 19**

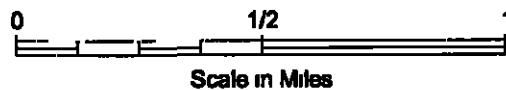
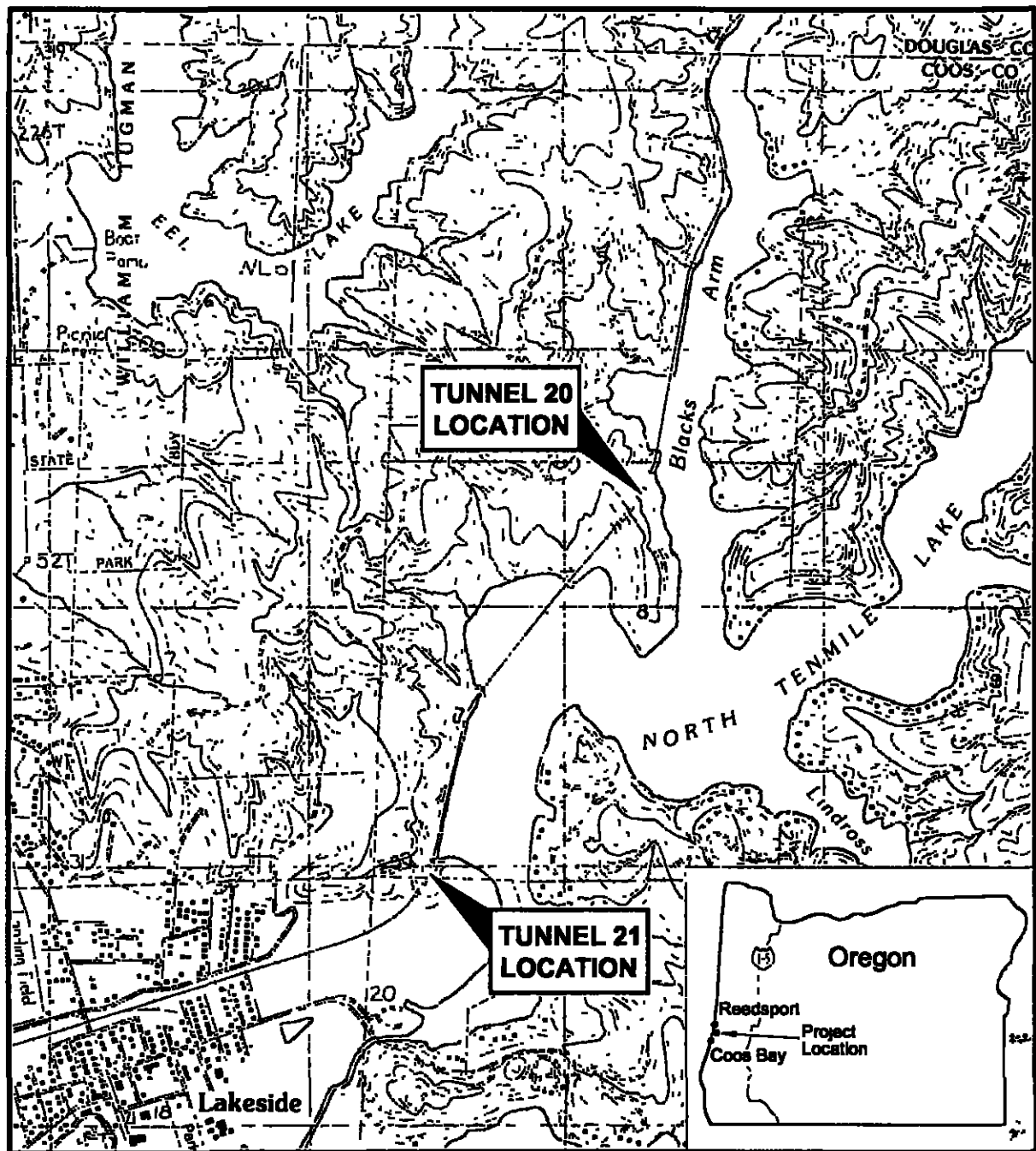
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**FIG. 7**





**NOTE**

Map adapted from 1:24,000 USGS topographic map of Lakeside, OR quadrangle, Provisional Edition, dated 1985.

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**VICINITY MAP TUNNELS 20 AND 21**

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**FIG. 8**